

May 20, 2024

Ref: 73305.01

Mr. Jeffrey Willis, Executive Director Coastal Resources Management Council Oliver H. Stedman Government Center 4808 Tower Hill Road Wakefield, RI 02879-1900

Re: May 28, 2024 Council Hearing Meeting

RIDOT – East Bay Bike Path Bridge Replacements – Reconstruction Barrington River Bridge (RIDOT Bridge No. 083751) and Palmer River Bridge (RIDOT Bridge No. 083851), Barrington and Warren, RI PTSID No. 0881A CRMC Application No. 2023-04-094

Dear Mr. Willis:

The Rhode Island Department of Transportation (RIDOT) submitted a Category B Assent Application on April 19, 2023 for proposed reconstruction of the East Bay Bike Path Bridges across the Barrington and Palmer Rivers in Barrington and Warren (the Project). The Project is scheduled to be heard at the May 28th full council hearing. The following person(s) will be representing the Project and will be available to answer questions and provide a presentation overview of the Project if the Council so desires:

- Lori Fisette / Loren Doyle (RIDOT) Project Introduction
- Andres Aveledo, PE (Aetna Bridge Company) Fishing Access
- Mark Costa, PE (VHB) Hydraulic Analysis

Thank you, and please feel free to contact me at (401) 457-2053 or aprezioso@vhb.com, if you have any questions or require additional information.

Sincerely, VHB

Andrew Prezioso, PE Project Manager, VHB

cc: Alisa Diaz Richardson, MS, PE, PMP, RIDOT Hamid Akinfolarin, Project Manager I, RIDOT Scott S. Hobson, PWS, VHB Andres Aveledo, PE, Project Manager, Aetna Bridge





May 16, 2024

Ref: 73305.01

Mr. Jeffrey Willis, Executive Director Coastal Resources Management Council Oliver H. Stedman Government Center 4808 Tower Hill Road Wakefield, RI 02879-1900

Re: Hydraulic Analysis of Barrington River adjacent to Atlantic Marine – Technical Memorandum

RIDOT – East Bay Bike Path Bridge Replacements – Reconstruction Barrington River Bridge (RIDOT Bridge No. 083751) and Palmer River Bridge (RIDOT Bridge No. 083851), Barrington and Warren, RI PTSID No. 0881A CRMC Application No. 2023-04-094

Dear Mr. Willis:

The Rhode Island Department of Transportation (RIDOT) submitted a Category B Assent Application on April 19, 2023 for proposed reconstruction of the East Bay Bike Path Bridges across the Barrington and Palmer Rivers in Barrington and Warren (the Project). More recently, under a cover letter dated May 3, 2024, initial results of a hydraulic and hydrologic study (Study) of the Barrington River adjacent to Atlantic Marine (Marina), were presented. The accompanying technical memorandum with supplemental reference appendices further describes the modeling approach and expands on the results of the Study.

As previously stated in the May 3 cover letter, RIDOT is committed to mitigate issues identified as it relates to the Marina, however, at this time, the data presented in the technical memorandum does not show that mitigation is warranted. The same cover letter dated May 3 also summarized discussions had with the Barrington Harbormaster regarding the improved safety of the river now with the removal of the many pile bents that supported the former trestle-style bridge. Attached to this cover letter is a letter from the Barrington Harbormaster to RIDOT stating his professional opinion regarding the improved safety of the river and showing his support of the project.

Thank you, and please feel free to contact either Ms. Alisa Diaz Richardson of RIDOT at (401) 479-1327 or Alisa.Richardson@dot.ri.gov, or me at (401) 457-2053 or aprezioso@vhb.com, if you have any questions or require additional information.

Sincerely, VHB

Andrew Prezioso, PE Project Manager, VHB

cc: Alisa Diaz Richardson, MS, PE, PMP, RIDOT Hamid Akinfolarin, Project Manager I, RIDOT Scott S. Hobson, PWS, VHB Andres Aveledo, PE, Project Manager, Aetna Bridge





TOWN OF BARRINGTON Rhode Island

Brian S. Hunt HARBORMASTER

04/29/2023

David Walsh State of RI DOT

Dear David,

I have carefully reviewed the current plan for the new bike path bridge and have come away with confidence that you have addressed all issues. The location of the Fishing pier on the north side of the bike path puts it out of the way of the mooring area, The overhead electric wires and the boat traffic which is a win win situation. The fact that the fishing pier is being designed to be ADA compliant brings it to a whole new level as something that has been needed in our area. The fishing pier will be accessible from the police cove parking lot which seems to have a lot of space and several handicap parking spots.

The design of the bridge with the additional rip rap being added to the east and west side will do a lot to address the whirlpool action of the water especially in the Atlantic marine area. The location of the navigable channel is located in a good location as well and will be much wider and higher which will make safe passage easier for boaters. The center set of pilings being surrounded by a rip rap will create a cradle type surround which will also reduce some of the effects of the current and whirlpool action.

I would like to summarize by congratulating all of those involved with the working redesign to try to accommodate everyone involved. I believe we have all done our due diligence in keeping everyone in the loop. I have spent many years on the water and specifically transiting from my home in Hundred Acre cove out to the bay and I honestly believe we have created a much safer easier passage way for boaters. I also believe that having a separate fishing pier will greatly enhance the safety of the bicycles using the bike path.

Sincerely, **Brian S Hunt**Brian S. Hunt
Harbormaster





Project #: 73305.01

Date: 05/15/2024

Shams Al-Amin

Re: East Bay Bike path - Hydrologic and Hydraulic Evaluation

Introduction

From: Mark Costa

The Rhode Island Department of Transportation (RIDOT) has contracted with the Aetna Bridge Company and VHB (via a joint venture) to complete a design build to remove and replace the East Bay Bike Path bridges over the Barrington River and Palmer River (the Project). Aetna Bridge recently completed Phase 1 of the Project in the summer of 2023 which demolished the existing bridges over the Barrington and Palmer Rivers. Phase 2 of the project is a proposed two-span bridge crossing (single pier) with abutments and a new independent fishing pier at each bridge location.

RIDOT has requested that Aetna Bridge and VHB complete a Hydrologic and Hydraulic (H&H) study to evaluate potential impacts on the abutting Atlantic Marine marina in Barrington due to replacing the former bridge crossing. The H&H study includes tasks for data collection and documentation review, bathymetric survey, hydrologic and hydraulic analysis, and development of a summary memorandum. This memorandum:

- Details the data collection including stream bathymetry and record flood studies;
- Provides details on the H&H analysis methodology;
- Presents results from the H&H analysis; and
- Provides summary conclusions.

All elevations noted in this memorandum are within the North American Vertical Datum of 1988 (NAVD 88).

Background Information

VHB reviewed relevant background information provided by RIDOT and publicly available. This included the following:

- Federal Emergency Management Agency (FEMA) Flood Insurance Study (FIS) and Flood Insurance Rate Map (FIRM). The currently effective FEMA FIRM is number 44001C0007H dated 7/7/2014, which shows the study area in coastal Zone AE with a Base Flood Elevation (BFE) of 12-feet (NAVD 88). (Appendix E)
- Report titled Hydraulic and Scour Analysis Barrington Bridge Prepared by WSP USA Inc., dated February 4, 2024. The goal of this report was to complete a scour analysis of the downstream State Route 114 Bridge. (Appendix F)
- Report titled *East Bay Bike Path Bridge Replacement Design Study Report* Bridge No. 083801 Prepared by AECOM, dated October 2014. (Appendix G)
- Plans titled Barrington Bridge No. 123 Prepared by Siegmund and Associates, Inc., dated June 2003. (Appendix C-2)



Ref: 73305.01 05/15/2024 Page 2



 Plans titled East Bay Bike Path Bridge Replacement Bridge Nos. 083751 & 083851 Volume 2 Prepared by VHB, dated 2022. (Appendix C-1)

The existing reports, plans, and modeling of the existing system were reviewed and incorporated into the analysis as described within.

Hydrologic and Hydraulic Analysis Methodology

The following sections describe methodology for the hydrologic and hydraulic analyses.

1.1. Hydrology - Riverine and Tidal Conditions

VHB completed a hydrologic analysis to estimate river flow for the Barrington River and Palmer River based on Rhode Island regional regression equations using the United States Geological Survey (USGS) StreamStats program. The USGS Streamstats Programs uses the technical guidance from the USGS Scientific Investigation Report 2014-5010 Equations for estimating selected streamflow statistics in Rhode Island, and the 50 Percent duration, or median, flows were used for each upstream boundary condition estimates to represent a normal riverine flow within the Barrington and Palmer River (Appendix A). This flow was inputted as a steady state flow of 41 cfs for the Barrington River, and 10.3 cfs for the Palmer River into the hydraulic model noted below.

VHB also evaluated the tide data for Providence Rhode Island based on National Oceanic and Atmospheric Administration (NOAA) Tidal Station ID 845400. This tidal gage is approximately 7.3 miles (as the crow flies) away from the Project site. This tidal gage was established in June of 1938, this location has two high-tides and two low-tides a day, and Table 1 provides the tidal datums based on a tidal epoch is from 1983-2001.

Table 1 NOAA Tidal Station 8454000 - Providence, RI

	Elevation
	(NAVD 88)
Mean High Water (MHW)	2.12
Mean Sea Level (MSL)	-0.22
Mean Low Water (MLW)	-2.29
Highest Astronomical Tide (HAT)	3.78

Stage hydrograph boundary conditions were developed to evaluate several tide scenarios over the tidal prism during approximately three (3) tide cycles. These stage boundary conditions were created using measured data from the NOAA Providence Tidal gage and include:

- Normal tide Represents an average tidal cycle in which the high and low tide elevations closely match the MHW and MLW tidal datums.
- Highest Astronomical Tide (HAT) Represents tidal conditions during a "Spring" or "King" tide in which the high tide elevations closely match the HAT tidal datum elevation
- 10-Year Storm Event A synthetic stage hydrograph was developed for the storm surge condition, based on the 10 percent annual chance Stillwater elevation from the FEMA FIS, transect 10 that is in the vicinity of the



Ref: 73305.01 05/15/2024 Page 3



project site. This was developed using a normal tide stage hydrograph and extrapolating one-tide to reach the 10-year stillwater FIS elevation.

Figure 1 shows the stage hydrographs used for different tidal boundary conditions in the model.

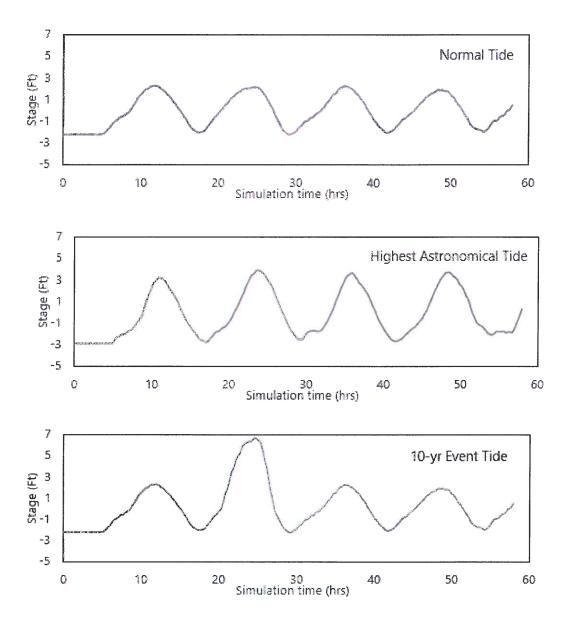


Figure 1: Stage Hydrographs for Different Boundary Conditions



Ref: 73305.01 05/15/2024 Page 4



1.2. Hydraulic Analysis

RIDOT provided VHB the previously developed United States Army Corps of Engineers (USACE) Hydrologic Engineering Center River Analysis System (HEC-RAS) model for East Bay Bike Path prepared by WSP to support their Barrington Bridge hydraulic and scour analysis. VHB reviewed this model, and used it as a base for the H&H analysis, and refined it to support this effort.

The updates and refinements of the previously completed modeling by WSP included:

- Expansion of the model domain area upstream (to the north) of the project site to incorporate the Hundred Acre Cove and the surrounding low-lying area, to fully capture the tidal prism.
- Location of the boundary conditions based on the updated model domain (locations shown in Figure 2).
 Boundary conditions were also updated to reflect flow input, normal, and Highest Astronomical Tide (HAT) conditions.
- Two-dimensional (2D) mesh, cell sizes, and cell locations throughout the modeling domain, with additional refinement and smaller cell sizes within the location of East Bay Bike path crossing, Route 103 crossing, and Atlantic Marine Marina to evaluate in greater detail the hydraulics of these areas.
- The East Bay Bike Path and downstream Berrington Bridge No. 123 were revised for the latest plans noted above and were modeled as SA-2D connection which allows flow to be modeled through the bridge structure and to be connected to the abutting 2D flow areas.
- Shallow Water Equations, Eulerian-Lagrangian Method (SWE-ELM) solver for 2D flow were used for model runs. Shallow water equations are recommended in HEC-RAS for tidally influenced conditions similar to the Project site.

VHB developed model geometries to represent the pre-development (condition as of early 2023), post-demolition (current condition in Spring 2024), and the post-development conditions with proposed East Bay Bike Path. VHB modeled a combination of geometries, riverine flows, and tide conditions for a total of up to nine (9) scenarios to allow comparison over a broad range of tidal events. Table provides a summary of the model scenarios for the H&H study.

Table 2: East Bay Bike Path Model Scenarios

Scenario #	East Bay Bike Path Bridge Condition	Barrington River Flow	Tidal Condition
1	Pre-Development	50-percent duration flow	Normal tide
2	Post-Demolition	50-percent duration flow	Normal tide
3	Post-Development	50-percent duration flow	Normal tide
4	Pre-Development	50-percent duration flow	Highest Astronomical Tide
5	Post-Demolition	50-percent duration flow	Highest Astronomical Tide
6	Post-Development	50-percent duration flow	Highest Astronomical Tide
7	Pre-Development	50-percent duration flow	10-percent annual chance tide
8	Post-Demolition	50-percent duration flow	10-percent annual chance tide
9	Post-Development	50-percent duration flow	10-percent ann lal cherce tide



Ref: 73305.01 05/15/2024 Page 5



1.2.1. Topo/Bathymetry

VHB subcontracted CR Environmental, Inc (CR) to complete a bathymetric survey of the Barrington River which was completed on March 12 and 13, 2024. The survey consisted of single beam echo sounding with precision navigation within the designated reach which spanned approximately 1,000 feet north of the East Bay Bike Path Bridge and 1,000 feet south of the Route 114/103 bridge and included detailed bathymetry of the Atlantic Marine Marina (Appendix B). A digital terrain model (DTM) was developed within HEC-RAS Mapper by combining the WSP terrain model and single beam bathymetric survey data 2024. The combined DTM has the horizonal datum of North American Datum of 1983 (NAD83) State Plane Coordinates Rhode Island (FIPS 3800) and vertical datum of North American Vertical Datum of 1988 (NAVD88). Units of both horizontal and vertical coordinate systems are in US survey feet. This bathymetry was compared to a previously collected bathymetry for the area and it was found to be generally consistent between the pre-development and post-demolition condition, therefore it was used in all model geometries.

1.2.2. Bridges and Crossings

The East Bay Bike Path bridge geometry for the pre-development scenarios (Scenarios #1, 4, and 7 listed above) and the location of piers and abutments were obtained from the bridge plans titled East Bay Bike Path Bridge Replacements Bridge NOS. 083751 & 083851 Reconstruction Plans Volume 2 (Appendix C-1). The bridge geometry for the post development model conditions (Scenarios #3, 6, and 9) were obtained from the plans East Bay Bike Path Bridge Replacement Bridge Nos. 083751 & 083851 Volume 2 prepared by VHB.

Bridge geometry for the downstream Bridge No. 123 were obtained from bridge plans titled *State Highway Replacement of Berrington Bridge No. 123* (Appendix C-2). Elevations in bridge plans are indicated in US feet measured in reference to the National Geodetic Vertical Datum of 1929 (NGVD29), which were converted into NAVD88 (NAVD88 = NGVD29 - 0.843 feet).

Pier locations were inputted directly into the terrain and with detailed 2D mesh modifications, and the bridge decks, low chords, and abutments were represented as a SA-2D connection in HEC-RAS which allows flow to be modeled through the bridge structure and to be connected to the abutting 2D flow areas.

1.2.3. Roughness

The following Manning's n values were assigned to landcover polygons imported to HEC-RAS using National Landcover dataset (USGS 2016). The National Landcover data set was further refined using aerial imagery in regions of open water, and marinas. A uniform *n* value of 0.025 was assigned throughout the Barrington River. Riprap around the pier in water and abutments of the proposed East Bay Bike Path Bridge was represented in the proposed conditions model as "riprap" landcover type with a Manning's *n* value of 0.036. Table 3 provides a summary of the Manning's n values used in HEC-RAS.



Ref: 73305.01 05/15/2024 Page 6



Table 3: Manning's n Values

Landcover Type	n Value
Open water	0.025
Developed, low intensity	0.040
Developed, high intensity	0.050
Developed, open space	0.025
Deciduous forest	0.100
Woody wetlands	0.100
Mixed forest	0.080
Pasture / hay	0.050
Evergreen forest	0.080
Cultivated crops	0.040
Grassland / herbaceous	0.045
Barren land / rock, sand, and clay	0.040
Emergent herbaceous wetlands	0.100
Scrub / shrub	0.070
Riprap	0.036



Ref: 73305.01 05/15/2024 Page 7



1.2.4. Model Domain and Computation Settings

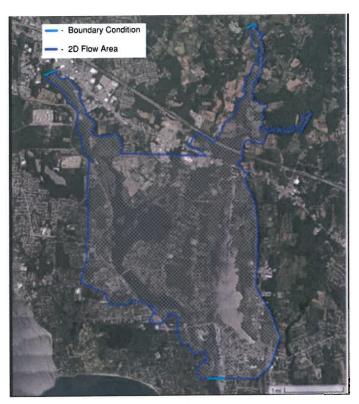


Figure 2 - Model Domain and Boundary Conditions

Figure 2 highlights the model domain of the study area with computational mesh and boundary conditions. A 50-ft by 50-ft cell size was used as a starting point for the two-dimensional (2D) flow area mesh. This was further refined around the East Bay Bike Path and Atlantic Marine marina area using breaklines and refinement regions to reduce the computational mesh cell size. Breaklines were established to preserve abrupt terrain changes at ridges, abutments, and piers aligning cell faces with high terrain elevations. Generally, a 15 ft x 15 ft cell size was used around the two bridges and the marina area, with further refinements up to 2 ft x 2 ft near the bridge piers. HEC-RAS version 6.4.1 was used to run the events. Shallow Water Equations, Eulerian-Lagrangian Method (SWE-ELM) solver for 2D flow options and PARDISO matrix solver were used. The model was run for unsteady flow computation.



Ref: 73305.01 05/15/2024 Page 8



H&H Modeling Results

VHB compared the flow velocities from the pre-development, post-demolition, and post-development condition between East Bay Bikepath crossing and the Route 113 crossing.

Normal Tide Conditions (Scenario 1 and 2)

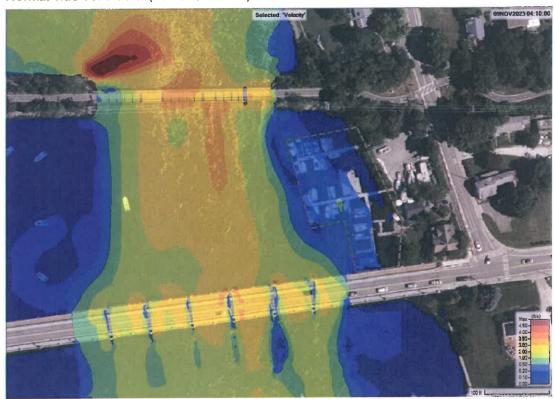


Figure 3 - Predevelopment Condition - Velocity - Peak of Outgoing Tide during a Normal Tide Cycle



Ref: 73305.01 05/15/2024 Page 9



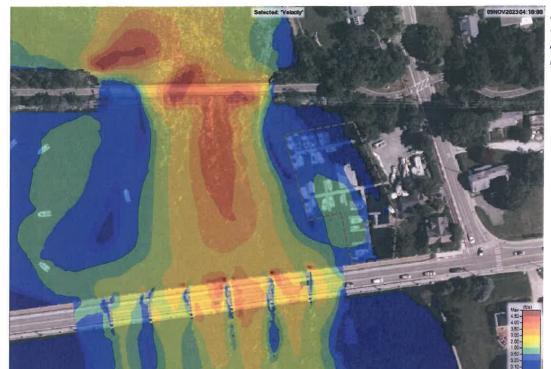
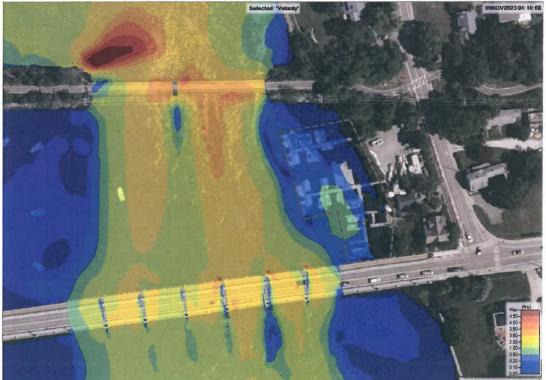


Figure 4 - Post-demolition Condition - Velocity - Peak of Outgoing Tide during a Normal Tide Cycle

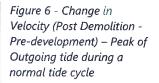


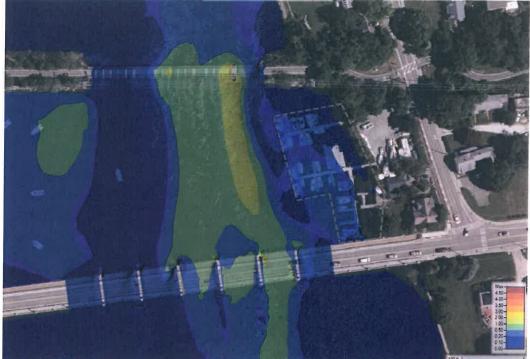
Development - Velocity Peak of Outgoing Tide
during a Normal Tide
Cycle



Ref: 73305.01 05/15/2024 Page 10







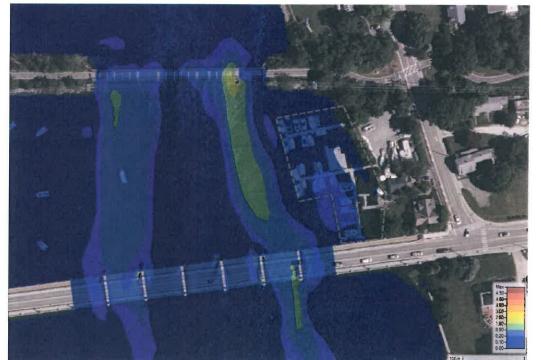


Figure 7 - Change in Velocity (Post Development - Pre-development) – Peak of Outgoing tide during a normal tide cycle



Rhode Island Coastal Resources Management Council (CRMC) Ref: 73305.01 05/15/2024 Page 11



The results of the modeling calculations indicate that there is an increase in flood velocities in the main channel of the Barrington River of approximately 1-3 feet/second during the peak of the outgoing tide during a normal tide cycle in the post-demolition and post-development condition in comparison to the pre-development condition. Within and along the perimeter of the Atlantic Marine marina flood velocities are generally consistent, noting that some internal areas are showing a maximum increase of flood velocity of approximately 0.4 feet/second in the post-demolition condition and 0.2 feet/second in the post-development condition in comparison to the pre-development condition during the peak of the outgoing tide during a normal tide cycle. The Atlantic Marine Marina area has a maximum velocity of approximately 1 foot/second in the pre-development, post-demolition, and post-development condition at the peak of a normal tide.

Abutting Marina Comparison

VHB also evaluated the modeling results for velocities at the nearby Stanley's Boat Yard and Barrington Yacht Club (located directly downstream of the Route 103 crossing over the Barrington River) and the Striper Marina (located southeasterly along the Palmer River). The location of each marina is shown on Figure 8.



Figure 8 - Locus of Abutting Marinas



Ref: 73305.01 05/15/2024 Page 12



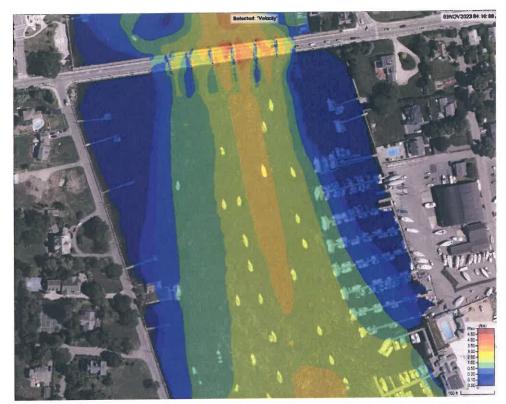


Figure 9 Post-demolition Condition -Velocity - Peak of Outgoing Tide during a Normal Tide Cycle at abutting Marinas

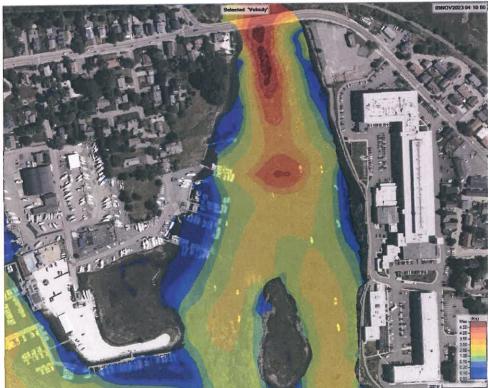


Figure 10 Post-demolition Condition -Velocity - Peak of Outgoing Tide during a Normal Tide (Striper Marina)



Ref: 73305.01 05/15/2024 Page 13



The H&H analysis calculates that the abutting Stanley's Boat Yard, Barrington Marina, and Striper Marina experience velocities of approximately greater than 1 foot/second along their perimeter, with a maximum velocity in the Barrington Marina of approximately 2 feet-second during a normal tide condition during the post-demolition condition (current condition).

Highest Astronomical Tide (Scenarios #4 +5)

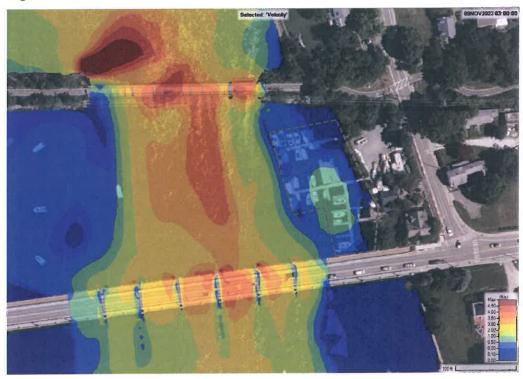


Figure 11 - Pre-development Condition - Velocity - Peak of Outgoing Tide during a Highest Astronomical Tide



Ref: 73305.01 05/15/2024 Page 14



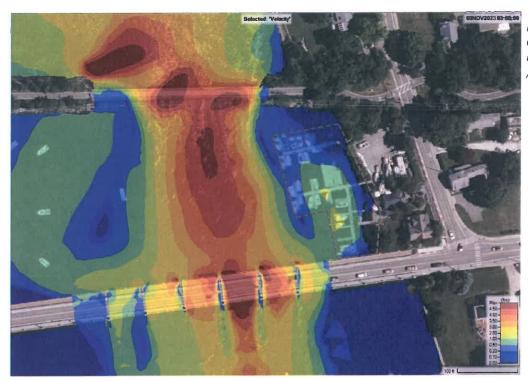


Figure 12 - Post-demolition Condition - Velocity - Peak of Outgoing Tide during a Highest Astronomical Tide

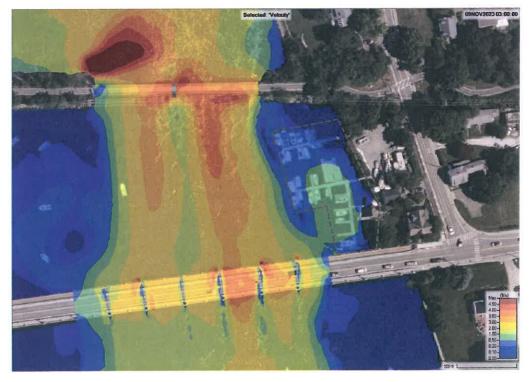


Figure 13 - Post-development Condition - Velocity -Peak Outgoing Tide during a Highest Astronomical Tide



Ref: 73305.01 05/15/2024 Page 15



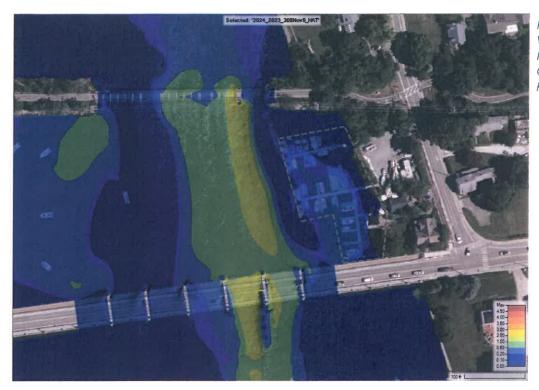


Figure 14 - **Change in**Velocity (Post Demolition Pre-development) - Peak **of**Outgoing tide during **a**highest astronomical **tide**

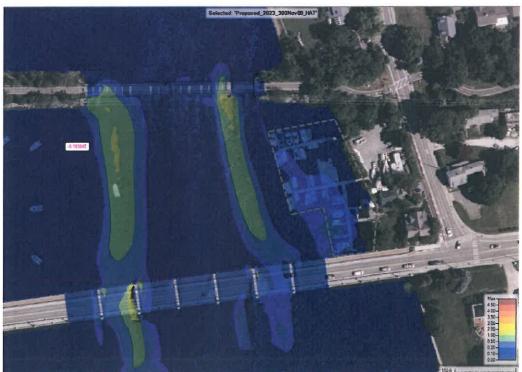


Figure 15 - Change in Velocity (Post Development - Predevelopment) - Peak of Outgoing tide during a highest astronomical tide



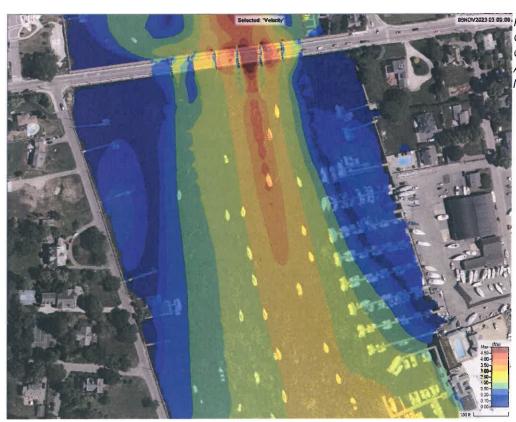
Ref: 73305.01 05/15/2024 Page 16



The results of the modeling calculations indicate that there is an increase in flood velocities in the main channel of the Barrington River of approximately 1-3 feet/second during the peak of the outgoing tide during a highest astronomical tide cycle in the post-demolition and post-development condition in comparison to the pre-development condition. Within and along the perimeter the Atlantic Marine marina flood velocities are generally consistent, noting that some internal areas are showing maximum increase of flood velocity of approximately 0.6 feet/second in the post-demolition condition and approximately 0.4 feet/second in the post-development condition during the peak of the outgoing tide during a highest astronomical tide cycle. The Atlantic Marine Marina has a maximum of velocity of approximately 1.1 feet/second on in the pre-development, post-demolition, and post-development condition at the peak of a highest astronomical tide.

Abutting Marina Comparison

VHB also evaluated the modeling results for velocities at the nearby Stanley's Boat Yard and Barrington Yacht Club (located directly downstream of the Route 103 crossing over the Barrington River) and the Striper Marina (located southeasterly along the Palmer River) during a Highest Astronomical Tide (HAT) during a post demolition conditions (current condition).



Condition - Velocity - Peak of
Outgoing Tide during a Highest
Astronomical Tide at abutting
Marina



Ref: 73305.01 05/15/2024 Page 17



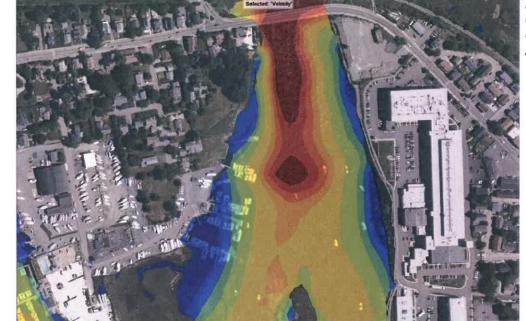


Figure 17 Post-demolition Condition - Velocity - Peak of Outgoing Tide during a Highest Astronomical Tide (Striper Marina)

COASTAL RESOURCES MANAGEMENT COUNCL

The H&H analysis calculates that the abutting Stanley's Boat Yard, Barrington Marina, and Striper Marina experience velocities of approximately greater than 1.5 foot/second along their perimeter, with a maximum velocity of approximately 2.9 feet-second during a highest astronomical tide condition during the post-demolition condition (current condition).

Conclusions

VHB analyzed a set of flow and tidal conditions using the two-dimensional H&H model and reviewed the velocities around the bridge and marina for this study.

The H&H model results indicate increases in river channel velocities in the main river channel abutting the Atlantic Marina Marine in both a normal tide and highest astronomical tide condition in the post-demolition and post-development condition. The modeling indicates that the Atlantic Marine Marina area have a maximum velocity of approximately 1 foot/second during the peak of a normal tide and 1.1 feet/second during the peak of a Highest Astronomical Tide during the pre-development, post-demolition, and post-development condition. The maximum velocity of the abutting Stanley's Boat Yard, Barrington Marina, and Striper Marina are approximately 2 feet/second and 2.9 feet/second during the peak of a normal tide and HAT, in the post-demolition condition. The modeling results indicate that the maximum velocities within the Atlantic Marine Marina area are less than those in the nearby abutting



May 3, 2024

Ref: 73305.01

Mr. Jeffrey Willis, Executive Director Coastal Resources Management Council Oliver H. Stedman Government Center 4808 Tower Hill Road Wakefield, RI 02879-1900

Re: Further Discussion of Fishing Structures

RIDOT – East Bay Bike Path Bridge Replacements – Reconstruction Barrington River Bridge (RIDOT Bridge No. 083751) and Palmer River Bridge (RIDOT Bridge No. 083851), Barrington and Warren, RI PTSID No. 0881A CRMC Application No. 2023-04-094

Dear Mr. Willis:

The Rhode Island Department of Transportation (RIDOT) submitted a Category B Assent Application on April 19, 2023 for proposed reconstruction of the East Bay Bike Path Bridges across the Barrington and Palmer Rivers in Barrington and Warren (the Project). More recently, design modifications were submitted under cover letters dated February 7, 2024 pertaining to fishing accommodations at both bridges and on April 18, 2024 for the fishing structure at the Barrington River bridge. The most recent modification shifted the location of the Barrington River fishing pier from the east side of the River to the west in response to public comments submitted to CRMC regarding the February 7, 2024 submission. RIDOT has provided descriptions of the site constraints and design elements associated with the proposed fishing accommodations in correspondence submitted previously, but this correspondence has been prepared to expand on RIDOT's goal of maintaining public fishing opportunities at each bridge location, consistent with Section 1.3.6(A) of the Coastal Resources Management Program (CRMP).

The New England Chapter Backcountry Hunters and Anglers has filed an objection to RIDOT's application describing what they perceive to be a significant decrease in the 'type and level' of public access for fishing, due to the lack of sidewalks on the proposed Bike Path bridges. RIDOT has previously provided information detailing environmental and site constraints which pose challenges and conflicts with other CRMC standards of attempting to widen the bridges. Widened bridges would be in direct conflict with construction methods needed to avoid the existing overhead wires and utility poles and would require a second bridge pier in each river channel to support the added weight. Additional encroachment into tidal waters and coastal wetlands would be required to accommodate wider bridge abutments, somewhat negating the work of the design team to minimize the Project's footprint on shoreline features to the greatest feasible extent.

Notwithstanding these challenges, RIDOT wishes to reiterate that the proposed height of the replacement bridges has been predicated upon vertical clearances dictated by the US Coast Guard (USCG) to match those of the Route 114 highway bridges immediately downstream – and by sound engineering practice to elevate the bridge superstructures above the 100-year wave velocity zones. The table below shows a comparison of the finish deck elevations of the proposed Bike Path bridges and the adjacent Route 114 roadway bridges, demonstrating conformance with the USCG's vertical clearance requirements.

	Proposed Bike Path Deck Elevation	Route 114 Bridge Sidewalk Maximum Elevation
Barrington River	15.95	15.59
Palmer River	14.20	14.35

This information is important in understanding the public benefit gained by widening the proposed bridges to provide sidewalks that could be considered redundant to the access already afforded by the Route 114 bridge sidewalks. Anglers are regularly observed fishing from the Route 114 bridge sidewalks, indicating that these are likely feasible and productive locations for catching fish. Furthermore, fishing from the increased height of the



Mr. Jeffrey Willis, Executive Director Ref: 73305.01 May 3, 2024 Page 2



proposed Bike Path bridges would bring the angler closer to the overhead utility lines and could pose conflicts with casting and fishing line entanglement. Adding sidewalks to the proposed Bike Path bridges would cause significant environmental, utility, schedule, and cost impacts to the project, without a commensurate level of increase in similar public fishing access.

In recognition that the sidewalks on Route 114 do not provide the same type of fishing access due to the increase in elevation (approximately 4.8 higher) when compared to the sidewalks on the former Bike Path bridges, RIDOT has included in the design a proposed fishing pier and bulkhead that will be built at elevations that more closely resemble the elevation of the former Bike Path bridges. The Barrington River fishing pier will be constructed with a deck elevation of 12.66, and the Palmer River bulkhead will have a finish grade of elevation 6.83. These proposed fishing accommodations will be distinct from each proposed bridge, yet in proximity. Entry to each fishing structure will occur off the Bike Path pavement, and each structure will be ADA accessible, in keeping with the remainder of the Bike Path, in contrast to the former bridges, which did not offer equal accessibility due to their elevated sidewalks that functioned as fishing positions. The structures have been designed in accordance with the United States Access Board's guide for *Accessible Fishing Piers & Platforms* to enhance public access for all anglers regardless of their mobility status. The proposed fishing structures are expected to avoid utility line conflicts and will provide more favorable heights in which to raise caught fish to the pier decks.

Since RIDOT's closure of the Bike Path bridges in 2019 for safety reasons, shore fishing along the banks of the Bike Path causeway, the adjacent shoreline, and Route 114 highway bridge sidewalks have continued to serve anglers as other feasible and productive locations for catching fish. As indicated in previous documentation, these areas will continue to be accessible to anglers following the reconstruction of the Bike Path bridges, and the Route 114 sidewalks will become fully available again once the temporary Bike Path detours are removed following completion of the bridge reconstructions. The proposed fishing accommodations are enhancements to these currently available fishing opportunities, and RIDOT feels that the provision of public fishing access has been met to the maximum extent practicable, as defined under CRMP § 1.1.2(A)(89), with the demonstration of constraints and addition of the separate fishing structures to the Project.

In consideration of CRMP § 1.3.6(A)(3) [It is the Council's policy to require applicants to provide, where appropriate, on-site access of a similar type and level to that which is being impacted as the result of a proposed activity or development project], similar types of fishing access will be accommodated on site in the proposed Project design. The proposed fishing structures will be constructed within the Project area adjacent to each bridge, will provide a fair length of edge from which to fish, are expected to maintain dropline, drift, and casting fishing methods, and built to a similar elevation as the historic bridges. Similarly, the added riprap required as scour protection along the Manmade Shoreline of the bridge abutments may increase shoreline fishing opportunities. Consistent with CRMP §§ 1.3.6(A)(2 and 3), the level of public access for fishing opportunities will be enhanced by the Project via the provision of accessible, ADA-compliant fishing accommodations, where none existed previously.

The design of the proposed bridges has been intensively evaluated and specifically honed for maximizing open water restoration and minimizing the footprint of shoreline impacts. RIDOT is offering fishing accommodations so that the loss of an important existing recreational public use will not occur. RIDOT recognizes that these accommodations may not satisfy all access features previously provided by the former Bike Path bridges, but the federal regulations, environmental, constructability, and profile constraints discussed in this application greatly limit the ability to do more. Additionally, RIDOT wishes to restate, as was emphasized in their September 13, 2023 correspondence, that it has no intention to prohibit, preclude, or restrict public recreational fishing from the proposed Bike Path Bridges.

Mr. Jeffrey Willis, Executive Director Ref: 73305.01 May 3, 2024 Page 3



Thank you, and please feel free to contact either Ms. Alisa Diaz Richardson of RIDOT at (401) 479-1327 or <u>Alisa.Richardson@dot.ri.gov</u>, or me at (401) 457-7824 or <u>shobson@vhb.com</u>, if you have any questions or require additional information.

Sincerely, VHB

Scott S. Hobson, PWS Senior Ecologist

cc: Alisa Diaz Richardson, MS, PE, PMP, RIDOT Hamid Akinfolarin, Project Manager I, RIDOT Andrew F. Prezioso, PE, VHB Andres Aveledo, Project Manager, Aetna Bridge

5:22 5. 2 John



May 3, 2024

Ref: 73305.01

Mr. Jeffrey Willis, Executive Director Coastal Resources Management Council Oliver H. Stedman Government Center 4808 Tower Hill Road Wakefield, RI 02879-1900

Re: Hydraulic Analysis of Barrington River adjacent to Atlantic Marine
RIDOT – East Bay Bike Path Bridge Replacements – Reconstruction
Barrington River Bridge (RIDOT Bridge No. 083751) and
Palmer River Bridge (RIDOT Bridge No. 083851), Barrington and Warren, RI
PTSID No. 0881A
CRMC Application No. 2023-04-094

Dear Mr. Willis:

The Rhode Island Department of Transportation (RIDOT) submitted a Category B Assent Application on April 19, 2023 for proposed reconstruction of the East Bay Bike Path Bridges across the Barrington and Palmer Rivers in Barrington and Warren (the Project). More recently, under a cover letter dated March 18, 2024, Coastal Resources Management Program (CRMP) compliance and Special Exception requests for Bridge Pier Riprap and fishing structures, along with considerations at Atlantic Marine (Marina), were presented. The considerations at the Marina included the design team for the Project conducting a hydrologic and hydraulic study (Study) of the area that includes the Marina, adjacent Barrington River, and the former bikeway bridge and existing highway bridge. The information provided in this letter and the associated appended figures is intended to provide an update on the Study commissioned by RIDOT that is ongoing to investigate the concerns of the Owners. Atlantic Marine is located southeast of the Barrington River bridge and has formally submitted comments indicating that the previous bridge helped attenuate flow velocities and direct strong currents away from the Marina.

Hydrologic and Hydraulic Study Overview

The Study is a full hydrologic and hydraulic (H&H) analysis of the Barrington River at the Marina (around the former bikeway bridge to the north and Route 103 highway bridge to the south) and surrounding estuary system including the upgradient Hundred Acre Cove. The Study uses numerical H&H modeling to evaluate the study area under 1) prior to bikeway bridge removal, 2) after bridge removal (current state), and 3) after the new bridge has been installed "proposed bridge model." The proposed bridge model includes all future riprap protection, in-water piles to support the new bridge pier, and in-water piles to support the proposed fishing pier off the northwest corner of the proposed bridge. Recent velocity measurements in December 2023 along the Barrington and Palmer Rivers, and at other marina locations along these rivers, were measured to assist and calibrate the H&H modeling. A full bathymetric survey of the Barrington River segment extending from just south of the highway bridge to just north of the former bikeway bridge was completed in March 2024 and has been incorporated into the H&H modeling.

Hydraulic Analysis Results

The results of the analysis to date have identified two key aspects of the flows in the Barrington River adjacent to, and within, the Marina.

First, the current state (with bridge demolition) and proposed bridge model analysis calculates that the velocities in the main channel of the river have increased in comparison to when the former bikeway bridge was in place. The proposed data provides a look at the flows in the river after the new bridge has been constructed. The new bridge will consist of a centralized pier that will be made up of ten piles supporting a concrete pier cap with mounded rip rap at its base and supplemental rip rap provided at each abutment. The Study model also incorporates the fishing pier founded on piles located off the northwest corner of the new bikeway bridge. The



Mr. Jeffrey Willis, Executive Director Ref: 73305.01 May 3, 2024 Page 2



construction of the new bridge and fishing pier in the river will help to mitigate the flows in the current state. The attached Figure 3 data chart from the Study shows a data point extracted from the model within the river channel and adjacent to the marina that indicate an increase of approximately 38% from 1.6 feet per second (fps) [0.95 knots] to approximately 2.2 fps [1.30 knots] during a normal outgoing tide event. The same data chart also indicates an outgoing tide increase of approximately 25% from 2.4 fps [1.42 knots] to 3.0 fps [1.78 knots] taken at the highest astronomical tide event. As a reference point, attached Figures 4, 5, and 6 provides velocity flow measurements taken in December 2023 on 3 days during the month that correlated closely with full moon and new moon tides of that month. At the time of these measurements, the former bikeway bridge had been demolished. The chart shows a snapshot in time and provides a comparative perspective of the river flows adjacent to the Marina during low (new moon) and high (full moon) tidal flow occurrences taken in the Barrington River and Palmer River confluence area.

• Second, the calculated velocities in the Marina are consistent with the velocities calculated prior to the removal of the former bikeway bridge and the current state and proposed bridge. The area defined as the Marina is the Marina Perimeter Limit (MPL), as defined by RI CRMP as an area extending a maximum of ten (10) feet outside of the marina structures (CRMP § 1.3.1(D)(9)(o). The model graphics attached show the river prior to bridge demolition and after the proposed bridge is constructed, reflecting that the flows within the MPL remain consistent with each other (see Figures 1 and 2). Attached Figure 4, 5, and 6 indicate that the Study model is accurate in showing that lower velocity flows are present within the MPL of the Marina in comparison to the river channel flows at various normal tide occurrences.

While RIDOT acknowledges the data indicates increases in river channel velocities in the main river channel, the modeling indicates that the velocities and flow patterns within the Marina area are consistent with the conditions prior to the bridge removal. These technical analysis results do not substantiate the claims that the MPL has been negatively impacted because of the bridge removal. RIDOT has stated that they are committed to mitigate issues identified as it relates to the Marina, however, at this time, mitigation within the Marina does not appear warranted. It has been noted in discussions with the Barrington Harbormaster that navigation through the Barrington River in the location of the former bridge is safer now with the elimination of the many pile bents that supported the former trestle-style bridge. In addition to the safety benefits provided by this Project, other benefits include better tidal flushing of the river and improved vertical clearance under the proposed bikeway bridge structure in comparison to the former bridge. It is our opinion that the calculated increase to velocity flow in the main river channel is outweighed by the benefits this Project provides to the community.

Thank you, and please feel free to contact either Ms. Alisa Diaz Richardson of RIDOT at (401) 479-1327 or <u>Alisa.Richardson@dot.ri.gov</u>, or me at (401) 457-2053 or aprezioso@vhb.com, if you have any questions or require additional information.

Sincerely, VHB

Andrew Prezioso, PE Project Manager, VHB

cc: Alisa Diaz Richardson, MS, PE, PMP, RIDOT Hamid Akinfolarin, Project Manager I, RIDOT Scott S. Hobson, PWS, VHB

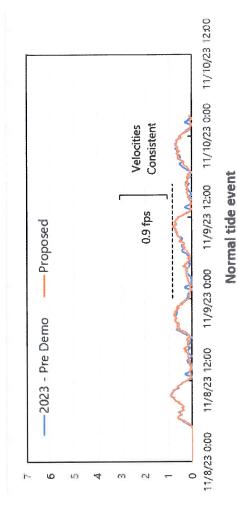
Andres Aveledo, PE, Project Manager, Aetna Bridge

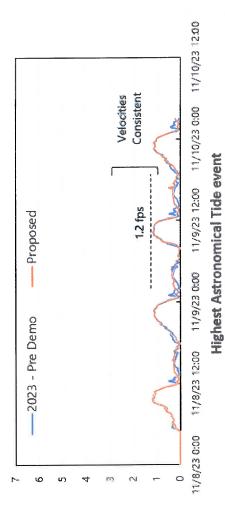


Figure 1: Comparison between 2023 (Pre-Demo), Proposed: Location 1 (at Marina)

COASTAL RESOURCES MANAGEMENT COUNCIL

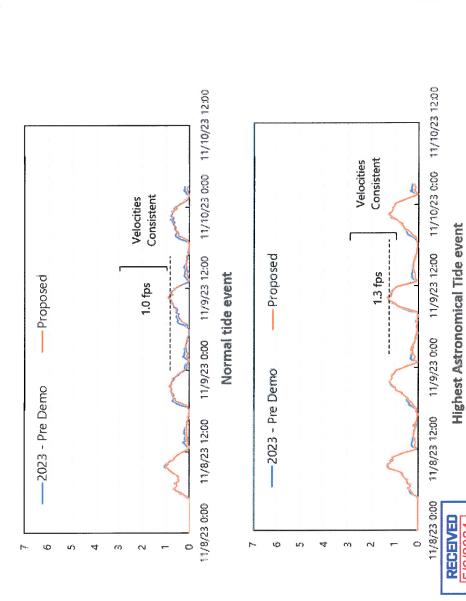
RECEIVED 5/6/2024

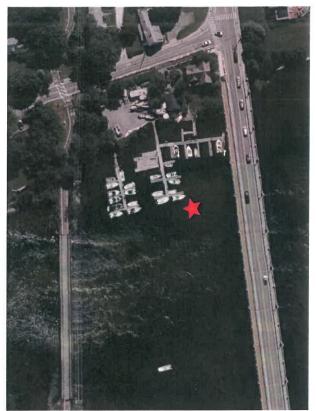




Location 1

Figure 2: Comparison between 2023 (Pre-Demo), Proposed: Location 2 (at Marina)



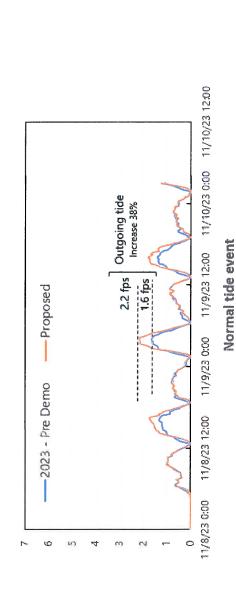


Location 2

COASTAL RESOURCES MANAGEMENT COUNCL

5/6/2024

Figure 3: Comparison between 2023 (Pre-Demo) and Proposed: Location 3 (River Channel)

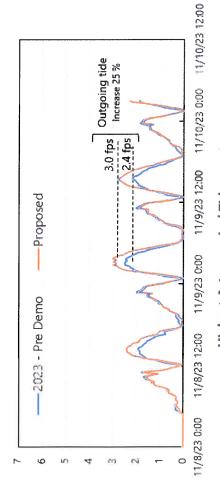


COABTAL RESOURCES MANAGEMENT COUNCE.

RECEIVED 5/6/2024



Location 3



Highest Astronomical Tide event



Figure 4: 12-01-2023 Flow Velocity Measurements Barrington, RI & Warren, RI



Velocity Measurement Locations

Location 1	ID.	Measurement Time	Velocity	(fns)
LUCALIUII I	עו	Measment I Illie	ACIOCICA	(IDS)

		(/
1	12:15 PM	1.6
2	12:20 PM	0.2
3	12:25 PM	2.8
4	12:30 PM	2.0
5	12:35 PM	2.0

Measurements taken at Locations 2, 3, and 4 correlate well with the Study model. Locations 2 and 4 in and adjacent to the Marina are lower than the main channel.

13	1:21 PM	1.8
14	1:30 PM	3.0

Full Moon: November 27, 2023 at 09:16 (GMT)



Figure 5: 12-14-2023 Flow Velocity Measurements Barrington, RI & Warren, RI



Velocity Measurement Locations

Location ID	Measurement Time	Velocity (fps)	
1	2:14 PM	1.4	
2	2:17 PM	0.2	
3	2:19 PM	1.2	l
4	2:20 PM	0.7	l
5	2:22 PM	1.8	

Measurements taken at Locations 2, 3, and 4 correlate well with the Study model. Locations 2 and 4 in and adjacent to the Marina are lower than the main channel.

13	2:57 PM	0.4
14	2:58 PM	0.4

New Moon: December 12, 2023 at 23:32 (GMT)



Figure 6: 12-28-2023 Flow Velocity Measurements Barrington, RI & Warren, RI



Velocity Measurement Locations

Location ID	Measurement Time	Velocity (fps)
1	10:12 AM	2.3
2	10:16 AM	0.7
3	10:14 AM	3.3
4	10:15 AM	0.9
5	10:18 AM	3.5

Measurements taken at Locations 2, 3, and 4 correlate well with the Study model. Locations 2 and 4 in and adjacent to the Marina are lower than the main channel.

13	10:45 AM	3.0
14	10:47 AM	3.8

Full Moon: December 27, 2023 at 00:33 (GMT)



March 15, 2024

Ref: 73305.01

Mr. Jeffrey Willis, Executive Director Coastal Resources Management Council Oliver H. Stedman Government Center 4808 Tower Hill Road Wakefield, RI 02879-1900

CRMP Compliance and Special Exception Request for Bridge Pier Riprap and Fishing Structure RIDOT - East Bay Bike Path Bridge Replacements - Reconstruction Barrington River Bridge (RIDOT Bridge No. 083751) and Palmer River Bridge (RIDOT Bridge No. 083851), Barrington and Warren, RI PTSID No. 0881A CRMC Application No. 2023-04-094

Dear Mr. Willis:

The Rhode Island Department of Transportation (RIDOT) submitted a Category B Assent Application on April 19, 2023 for proposed reconstruction of the East Bay Bike Path Bridges across the Barrington and Palmer Rivers in Barrington and Warren (the Project). More recently, design modifications were submitted under cover letter dated February 7, 2024 pertaining to fishing accommodations and riprap scour protection at each bridge pier. The February submission presented the design and need for each proposed modification, while this current submission addresses Coastal Resources Management Program (CRMP) compliance aspects of the modifications, as requested by CRMC permitting staff, and expands on the efforts that RIDOT has undertaken to address concerns raised at the public hearing held before the full Council on October 24, 2023. Those concerns focused on maintaining public fishing opportunities and reviewing concerns associated with the potential increase in flows and current velocities at Atlantic Marine, located southeast of the Barrington River bridge.

Pier Riprap and Palmer River Fishing Accommodation

As indicated in VHB's February 7, 2024 correspondence, riprap scour protection at the single pier for each bridge was not initially designed into the Project but was determined necessary during final design, as the effects of prolonged local scour and extreme storm events were studied further. Consequently, RIDOT is proposing the installation of stone riprap to adequately protect the bridge piers from these events. Similarly, designated fishing accommodations were not initially proposed, but are now provided in response to public request, and RIDOT considers the existing fill bulkhead on the Palmer River to be a suitable location for public fishing access. The proposed filling in tidal waters required to accomplish each of these Project elements falls under CRMP § 1.3.1(J) -Filling in Tidal Waters. It should be noted that piles generally are not considered to be "fill" under Section 404 of the Clean Water Act, and it is RIDOT's understanding that installation of the proposed steel micropiles to support the timber fishing pier at the Barrington River does not constitute filling in tidal waters under the CRMP.

CRMP Compliance

In accordance with the Policies listed at CRMP §§ 1.3.1(J)(1)(a, c, e, and f), RIDOT understands that filling in Tidal Waters is discouraged and that filling is to be minimized when it is necessary. The filling currently proposed in Tidal Waters is the minimum necessary to achieve the stated Project goals and will be accomplished exclusively for public use and benefit. The pier riprap will protect the structural integrity of the two public bridges to be reconstructed, and the fishing pier is proposed to provide public access to tidal waters. By virtue of the Project type, all facets of the Project are believed to comply with CRMP § 1.3.6 concerning public access.

In consideration of the Prerequisites listed at CRMP § 1.3.1(J)(2)(a and b), at a federal level, the Project falls under RI General Permit 8, and the bridge reconstruction was authorized as such by the US Army Corps of Engineers (USACE)

P 401,272,8100



Mr. Jeffrey Willis, Executive Director Ref: 73305.01 March 15, 2024 Page 2



on September 8, 2023 (USACE File Number NAE-2022-02797). Given that the Project was authorized under the RI General Permits, it is RIDOT's understanding that water quality certification under Section 401 of the Clean Water Act is granted conditionally by the RI Department of Environmental Management (RIDEM) under Water Quality Certificate No. 22-011. RIDOT held a consultation meeting with USACE regulatory staff on February 9, 2024 to discuss the pier riprap and fishing accommodations, and it was confirmed that the new fills will still meet the thresholds of the RI General Permits. The pier riprap and Palmer River fishing bulkhead will be permitted as modifications to the initial permit under RI General Permit 8, such that Section 401 water quality certification will continue to be granted conditionally. The timber fishing pier proposed at the Barrington River will be filed as a Pre-Construction Notification application under RI General Permit 4, and Section 401 water quality certification will again be granted conditionally. RIDOT understands that a CRMC Assent is not valid until all required USACE authorizations have been received. Furthermore, the Project is being reviewed for State Water Quality Certification by RIDEM for compliance with the RI Water Quality Regulations (250-RICR-150-05-1).

Filling in Tidal Waters to accomplish scour protection at the piers is proposed at the interface of Type 2 and 3 Waters, and filling required for the fishing accommodation at the Palmer River is proposed in Type 2 Waters – where the Water types are defined under CRMP §§ 1.2.1(C and D) and the Prohibitions are stated under CRMP § 1.3.1(J)(3). Riprap proposed at the bridge piers will be subtidal, such that effects to the scenic value of Type 2 Waters are believed not pertinent (CRMP § 1.2.1(C)(2)(a)), and fills associated with the fishing bulkhead will be low in profile and are not expected to negatively impact shoreline scenic value. Additional stone riprap is proposed at the existing bulkhead to form a more aesthetically pleasing north (seaward) face where sheet piling is not present, and additional riprap scour protection proposed against the sheet piling along the bulkhead's east (seaward) side will be predominantly subtidal. Filling in coastal wetlands is not proposed (CRMP § 1.3.1(J)(3)(b)).

The East Bay Bike Path is a well-established, heavily used public facility that RIDOT considers to be a priority use for the area (CRMP § 1.3.1(J)(3)(c)(1). It enables physical and visual shoreline access for statewide benefit. The filling is believed necessary to ensure the long-term structural integrity of each bridge and to preserve an existing public use in the form of fishing. RIDOT has examined scour protection alternatives (CRMP § 1.3.1(J)(3)(c)(2)), and hydraulic calculations conclude that riprap is required to the proposed depth and lateral dimensions to minimize unacceptable deflection of piles comprising the piers (CRMP § 1.3.1(J)(3)(c)(3)). RIDOT similarly has studied alternatives for fishing locations and has conducted field reviews to assess potential locations and observe active fishing. The location of the existing bulkhead was considered by RIDOT to be an appropriate location with sufficient seaward reach. As such, the need for construction of a separate fishing pier was not identified, and a location at any other bridge quadrant would likely require encroachment within coastal wetlands (salt marsh). The area and volume of proposed riprap installation in Tidal Waters at each bridge and at the fishing bulkhead are presented in Revised Tables 4-1 and 4-2, accompanying the previously filed February 7, 2024 correspondence.

In consideration of CRMP § 1.3.1(J)(4), it is RIDOT's understanding that the Project will not be assessed fees for fills in Tidal Waters (CRMP § 1.1.12), given that the State is the Applicant (and Project proponent) and that no application fees were assessed for the initial filing.

Special Exception Request - CRMP § 1.1.8

RIDOT respectfully requests the Council's consideration for the granting of a Special Exception for proposed filling in Tidal Waters, as required for installation of riprap scour protection on the riverbed of both rivers and for converting the existing construction bulkhead to a permanent fishing structure on the shoreline of the Palmer River. The activities requiring a Special Exception are described above and may constitute prohibited activities under CRMP § 1.3.1(J)(3). The activities are believed to be the minimum needed to achieve the goals of providing adequate scour protection for the bridge piers and of restoring designated public fishing access to the Palmer River within the Project area.

In consideration of CRMP § 1.1.8(A)(1)(a and c), the Project is believed to serve a compelling public purpose and provides a public benefit. The activities support the restoration of actively used public, transportation infrastructure.



Mr. Jeffrey Willis, Executive Director Ref: 73305.01 March 15, 2024 Page 3



The Bike Path is believed to be considered a priority use for Type 2 Waters, has been a long-standing public use abutting Type 3 Waters, and provides physical and visual access to the shoreline for broad segments of the public.

In consideration of CRMP § 1.1.8(A)(2), all reasonable steps are believed to have been taken to minimize environmental impacts and use conflicts. The filling will occur where saltmarsh and other special aquatic sites are absent and where use conflicts will be avoided. For example, coordination has been maintained with the Town of Barrington Harbormaster to ensure that placement of riprap scour protection under the western edge of the Barrington River navigation channel will not pose a conflict with mariners, while the fishing pier is being proposed to avoid potential use conflicts on a replacement bridge of reduced width and to prevent a potential reduction of a popular public use via the creation of designated public fishing access. To ensure Project compatibility with marine resources, RIDOT held a joint consultation meeting with NOAA Fisheries Protected Resources Division, NOAA Fisheries Habitat Conservation Division, and RIDEM's Division of Marine Fisheries on February 26, 2024 specific to the recently proposed tidal fills. Relevant Project documentation was subsequently submitted to NOAA Fisheries, and results of the consultations are attached.

In consideration of CRMP § 1.1.8(A)(3), the pier riprap is required based on hydraulic scour studies conducted during final design. The reduction from two bridge piers to one in the Design-Build Entity's Project design has reduced in-water disturbances and riprap fills in Tidal Waters to a single pier location at each river, and pier locations will be set within the alignment of the former bridges. An alternative proposing no riprap could jeopardize the stability of the bridges in large storm events and over time. The proposed conversion of a temporary bulkhead to a permanent fishing pier was decided upon for its location at minimizing environmental impacts and for providing a location that RIDOT perceives to be readily accessible and suitable for casting to varying water depths. Alternatives proposing a fishing structure at any of the other three bridge quadrants would likely require encroachment within or over coastal wetlands, and potentially in Type 1 Conservation Waters, as defined by CRMP § 1.2.1(B).

With respect to CRMP § 1.1.8(C), the filling in Tidal Waters would be a one-time event during construction, and the fill type and dimensions have been specified on the Project drawings. The Contractor would install the fills in accordance with the approved Project drawings.

Considerations Related to Atlantic Marine

In accordance with the Council's request at the October 24, 2023 hearing regarding the potential increase in tidal flow and velocity at Atlantic Marine, RIDOT has been in communication with the Marina Owners, has been in contact with the Barrington Harbormaster, and has begun initial steps to better understand the concerns of the Owners. The Marina Owners have indicated that the previous bridge helped attenuate flow velocities and direct strong currents away from the Marina. In the time since the Council hearing, RIDOT has set out to collect facts and characterize the Marina's concern.

This is being accomplished by performing a full hydrologic study (Study) of the Barrington River at the Marina and surrounding area (around the former bikeway bridge to the north and Route 103 highway bridge to the south). This Study will model the area prior to bikeway bridge removal, subsequent to bridge removal (current state), and after the new bridge has been installed. The "new bridge" model will include future riprap protection, in-water piles comprising the bridge pier, and in-water piles to support the proposed fishing pier. Of note, it is RIDOT's intention that the piles supporting the proposed fishing pier assist in trapping and deflecting ice and debris flows on the Marina side of the channel on outgoing tides. A comparative assessment to be conducted will provide RIDOT with data driven guidance to determine next steps. As part of the Study, RIDOT has initiated preliminary velocity capture measurements along the Barrington and Palmer Rivers, and at other marina locations along these rivers, to assist with the hydraulic modeling. A full bathymetric survey of the Barrington River segment extending from just south of the highway bridge to just north of the former bikeway bridge has been completed. Additionally, it has been noted that tides in general have been elevated, exacerbating flows through the Barrington River. Tide characteristics will be studied further, and NOAA tide data relevant to this observation will be included in the Study.



Mr. Jeffrey Willis, Executive Director Ref: 73305.01 March 15, 2024 Page 4



The Study is a necessary first step to characterize and understand the hydrology with respect to the project area and Marina. RIDOT is committed to mitigate any issues identified when the final engineering analysis is complete and needs to fully evaluate how any proposed mitigative measure might affect other portions of the channel, as well as existing public and private infrastructure. It has been noted in discussions with the Barrington Harbormaster that navigation through the River in the location of the former bridge is safer now with the elimination of the many pile bents that supported the former trestle-style bridge. Further of note, it is our understanding that mariner travel through the waters at this location was a challenge prior to the bridge removal and will remain subjective to the boater's experience. It is anticipated that early results of the Study will be available to share in presentation format at the next full Council hearing, with a comprehensive finalized report following thereafter.

Thank you, and please feel free to contact either Ms. Alisa Diaz Richardson of RIDOT at (401) 479-1327 or Alisa.Richardson@dot.ri.gov, or me at (401) 457-7824 or shobson@vhb.com, if you have any questions or require additional information.

Sincerely, VHB

Scott S. Hobson, PWS Senior Ecologist

5 M S. Ada

cc: Alisa Diaz Richardson, MS, PE, PMP, RIDOT Hamid Akinfolarin, Project Manager I, RIDOT Andrew F. Prezioso, PE, VHB

Andres Aveledo, Project Manager, Aetna Bridge

NOAA Fisheries GARFO PRD Updated ESA Section 7 Consultation



This page intentionally left blank.



Appendix A. Verification Form (updated December 10, 2020)

Project Activity Type (check all that apply to the entire action):

Federal Highway Administration (FHWA) or the applicable state Department of Transportation (DOT) shall submit a signed version of this completed form, together with any project plans, maps, supporting analyses, etc., to NOAA's National Marine Fisheries Service (NMFS), Greater Atlantic Regional Fisheries Office, Protected Resources Division (GARFO PRD) at nmfs.gar.esa.section7@noaa.gov with "FHWA GARFO NLAA Program: [Project Title or Number]" in the subject line. Note: project design contractors and/or consultants may assist in preparing the form, but only FHWA/DOT staff shall sign off on it on the final page.

1. Bridge repair, demo	placement project rway access project (incl	•	demolition, and repairs)
Transportation Project	Information		
Name of Project:	Barrington and Warren East	Bay Bike Path Bridges	
Reinitiation (Yes/No):	Yes		
State DOT/Program:	RIDOT		
DOT ID Code:	0188A		H
Contact Person:	Nicole Lineberry		
Phone:	401-497-1327	Email:	nicole.leporacci@dot.ri.gov
Project Latitude (e.g., 42.0	625884):	41.737654	
Project Longitude (e.g., -7		-71.295501	
Maximum Water Depth (1	n)	6.0	
Anticipated Project Start	6/1/2024	Anticipated	C/4/000C
Date:	0/1/2024	Project End Date:	6/1/2026
City/Town:	Barrington and Warren	Water body:	Barrington and Palmer River
Project/Action Description and Purpose:	Bay Bike Path (EBBP) over the which carries the East Bay Barrington. This form include each other and that the proplecation. This form has been of project modifications to the consultation was completed.	the Palmer River in War Bike Path (EBBP) over the es both bridges due to the osed activities and impa completed for a re-initial e construction of the brid in February 2022, both loof work and impacts in s	ne proximity of the bridges to acts are the same at each ation of consultation as a result
	be four spuds with an impact raised and lowered into the r for a short period of time befoliocation. There will be no implayed will be installed from s preclude use of the substrate upon the removal of the spuda mixture of sand/hard bottom. The construction of a single	t of 1 sq. ft each. These iver bottom and disturb ore being raised and the pact or vibratory hamme hore at the bridges. The surface by aquatic orgons. This will result in a sum.	the surface of the river bottom be barge moved to a new or used to drive piles. The temporary impact will be to anisms but that use will return ub-total impact of 40 sq. feet to



	listed species and/or critical habitats in the			
√	Atlantic sturgeon (all DPSs)		Kemp's ridley sea	
	Atlantic sturgeon critical habitat Indicate which DPS (GOM, NYB, Chesapeake Bay DPSs):		Loggerhead sea to (Northwest Atlan	
	Select DPS			
√	Shortnose sturgeon		Leatherback sea t	urtle
	Atlantic salmon (GOM DPS)		North Atlantic rig	tht whale
	Atlantic salmon critical habitat (GOM DPS)		North Atlantic rig critical habitat	tht whale
	Green sea turtle (North Atlantic DPS)		Fin whale	
The	rmation for your action area at: https://www.fisher.ntic/consultations/section-7-species-critical-habitate following stressors are applicable to the action of the section	ies.no	aa.gov/new-england-	-mid-
	acts Table			
Habi	tat Alteration			
		F	Permanent (acres)	Temporary (acres)
	(saline)		0.38	0.00
Silt/N	Mud/Clay (saline)			
Hard	bottom (saline)		0.38	
Subm	1 A CONTRACTOR OF THE STATE OF		0.00	0.00
	nerged Aquatic Vegetation (SAV) (saline)		0.00	0.00
	(freshwater)		0.00	0.00
Sand			0.00	0.00
Sand Silt/N	(freshwater)		0.00	0.00
Sand Silt/N Hard	(freshwater) Mud/Clay (freshwater)		0.00	0.00
Sand Silt/N Hard	(freshwater) Mud/Clay (freshwater) bottom (freshwater)		0.00	0.00
Sand Silt/N Hard Subm	(freshwater) Mud/Clay (freshwater) bottom (freshwater)	0.38		0.00
Sand Silt/N Hard Subm	(freshwater) Mud/Clay (freshwater) bottom (freshwater) nerged Aquatic Vegetation (SAV) (freshwater) l amount of habitat alteration	0.38		0.00
Sand Silt/N Hard Subm	(freshwater) Mud/Clay (freshwater) bottom (freshwater) nerged Aquatic Vegetation (SAV) (freshwater)			0.00
Sand Silt/N Hard Subm Total	(freshwater) Mud/Clay (freshwater) bottom (freshwater) nerged Aquatic Vegetation (SAV) (freshwater) I amount of habitat alteration atter Construction Impacts		nount in meters	
Sand Silt/M Hard Subm Total In-wa	(freshwater) Mud/Clay (freshwater) bottom (freshwater) nerged Aquatic Vegetation (SAV) (freshwater) l amount of habitat alteration atter Construction Impacts h of water body in action area (m)			
Sand Silt/M Hard Subm Total In-wa Widtl Stress	(freshwater) Mud/Clay (freshwater) bottom (freshwater) nerged Aquatic Vegetation (SAV) (freshwater) I amount of habitat alteration atter Construction Impacts		nount in meters	.0



Project Design Criteria (PDC) Checklist

FHWA/DOT shall incorporate all general PDCs and all applicable PDCs in the appropriate stressor categories. For any PDCs that are not incorporated, additional justification is required for a project to be eligible for the NLAA Program. FHWA/DOT shall check the corresponding box for each PDC that is, or will be, incorporated into the project or indicate if not applicable.

GEN	ERAL	PDCs	
Yes	N/A	PDC#	PDC Description
✓		1.	Ensure all operators, employees, and contractors are aware of all FHWA environmental commitments, including these PDC, when working in areas where ESA-listed species may be present or in critical habitat.
V		2.	No portion of the proposed action will individually or cumulatively have an adverse effect on ESA-listed species or critical habitat.
	V	3.	No portion of the proposed action that may affect the GOM DPS of Atlantic salmon will occur in the tidally influenced portion of rivers/streams where their presence is possible from April 10 through November 7. The range of the GOM DPS only occurs in Maine. Note: If the project will occur within the geographic range of the GOM DPS Atlantic salmon but their presence is not expected following the best available commercial scientific data, the work window does not need to be applied. Please attach best available information (i.e. local fisheries biologist correspondence).
		4.	No portion of the proposed action that may affect shortnose or Atlantic sturgeon will occur in areas identified as spawning grounds as follows: i. Gulf of Maine: Apr 1-Aug 31 ii. Southern New England/New York Bight: Mar 15-Aug 31 iii. Chesapeake Bay: Mar 15-Jul 1 and Sep 15-Nov 1 Note: If river specific information exists that provides better or more refined time of year information, those dates may be substituted with NMFS approval.
	√	5.	No portion of the proposed action that may affect shortnose or Atlantic sturgeon will occur in areas identified as overwintering grounds where dense aggregations are known to occur as follows: i. Gulf of Maine: Oct 15-Apr 30 ii. Southern New England/New York Bight: Nov 1-Mar 15 iii. Chesapeake Bay: Nov 1-Mar 15
	√	6.	Note: If river specific information exists that provides better or more refined time of year information, those dates may be substituted with NMFS approval. Within designated critical habitat for Atlantic sturgeon, no work will affect hard bottom substrate (e.g., rock, cobble, gravel, limestone, boulder, etc.) in low salinity waters (i.e., 0.0-0.5 parts per thousand) (PBF 1).
✓		7.	Work will result in no or only temporary/short-term changes in water temperature, water flow, salinity, or dissolved oxygen levels.



Yes	N/A	PDC #	PDC Description
√		8.	If ESA-listed species are (a) likely to pass through the action area at the time of year when project activities occur; and/or (b) the project will create an obstruction to passage when in-water work is completed, then a zone of passage (~50% of water body) with appropriate habitat for ESA-listed species (e.g., depth, water velocity, etc.) must be maintained (i.e., physical or biological stressors such as turbidity and sound pressure must not create barrier to passage).
V		9.	The project will not adversely impact any submerged aquatic vegetation (SAV) or oyster reefs.
\checkmark		10.	No blasting or use of explosives will occur.
✓		11.	No in-water work on large dams or tide gates (small dam and tide gate repairs may be permitted with prior review and approval from NMFS).

UNDE	ERWA	TER NO	DISE PDCs
Yes	N/A	PDC#	PDC Description
		12.	If pile driving is occurring during a time of year when ESA-listed species may be present, and the anticipated noise is above the behavioral noise threshold, a "soft start" is required to allow animals an opportunity to leave the project vicinity before sound pressure levels increase. In addition to using a soft start at the beginning of the work day for pile driving, one must also be used at any time following cessation of pile driving for a period of 30 minutes or longer. For impact pile driving: pile driving will commence with an initial set of three strikes by the hammer at 40% energy, followed by a one minute wait period, then two subsequent three-strike sets at 40% energy, with one-minute waiting periods, before initiating continuous impact driving. For vibratory pile installation: pile driving will be initiated for 15 seconds at reduced energy followed by a one-minute waiting period. This sequence of 15 seconds of reduced energy driving, one-minute waiting period will be repeated two additional times, followed immediately by pile-driving at full rate and energy.



Yes	N/A	PDC#	PDC Description
	✓	13.	If the project includes non-timber piles*, please attach your calculation to this verification form showing that the noise is below the injury thresholds of ESA-listed species in the action area. The GARFO Acoustic Tool can be used as a source, should you not have other information: https://www.fisheries.noaa.gov/new-england-mid-atlantic/consultations/section-7-consultation-technical-guidance-greater-atlantic.
			*Effects from timber and steel sheet piles were analyzed in the NLAA programmatic consultation, so no additional information is necessary.
✓		14.	Any new pile-supported structure must involve the installation of no more than 50 piles (below MHW).

Pile material (e.g., steel pipe, concrete)	Pile diameter/ width (inches)	Number of piles	Installation method (e.g., impact hammer, vibratory start and then impact hammer to depth, drilling)
steel micropiles (bike path	12	20	Drilling
steel micropile (fishing pier)	24	18	Drilling

IMPI	NGEM	ÆNT/EN	TRAINMENT AND ENTANGLEMENT PDCs
Yes	N/A	PDC#	PDC Description
	✓	15.	If excavating or dredging, only mechanical buckets, hydraulic cutterheads, or low volume hopper dredges (e.g., CURRITUCK, ≤300 cubic yard maximum bin capacity) may be used. Note: We consider excavating a smaller scale form of mechanical dredging.
	✓	16.	No new excavation or dredging in Atlantic sturgeon or salmon critical habitat (excavation in a prior construction footprint or maintenance dredging is permitted, but still must meet all other PDCs). New excavation or dredging outside Atlantic sturgeon or salmon critical habitat is limited to one-time events (e.g., burying a cable or utility line) and minor (≤2 acres) expansions of areas already subject to prior excavation or maintenance dredging. Locating a replacement bridge within 250 feet (centerline to centerline) of an existing bridge and excavation of sediment around bridge piers are considered work in a previous construction footprint. Note: We consider excavating a smaller scale form of mechanical dredging.



Yes	N/A	PDC#	PDC Description
	✓	17.	Temporary intakes related to construction are prohibited in sturgeon and salmon spawning, rearing, or overwintering habitat during the time of year windows identified in General PDCs 3-5. If utilized outside those areas and times of year and in an area with anticipated sturgeon and salmon presence, temporary intakes must be equipped with 2-millimeter wedge wire mesh screening and must not have greater than 0.5 feet per second intake velocities, to prevent impingement or entrainment of juvenile and early life stages of these species.
	✓	18.	Work behind cofferdams, turbidity curtains, or other instruments that prevent access of animals to the project area is required when ESA-listed species are likely to be present (if presence is limited to rare, transient individuals, access control measures are not necessary). Once constructed, work inside a cofferdam at any time of year may be permitted with NMFS approval, provided the cofferdam is installed/removed outside the time-restricted period.
\checkmark		19.	No new permanent surface water withdrawal, water intakes, or water diversions.
✓		20.	Turbidity control measures, including cofferdams, must be designed to not entangle or entrap ESA-listed species.
	✓	21.	Any in-water lines, ropes, or chains must be made of materials and installed in a manner to minimize or avoid the risk of entanglement by using thick, heavy, and taut lines that do not loop or entangle. Lines can be enclosed in a rigid sleeve.

WAT	WATER QUALITY/TURBIDITY PDCs					
Yes	N/A	PDC #	PDC Description			
	✓	22.	In-water offshore disposal may only occur at designated disposal sites that have already been the subject of ESA section 7 consultation with NMFS and where a valid consultation is in place.			
√		23.	Any temporary discharges must meet state water quality standards (e.g., no discharges of substances in concentrations that may cause acute or chronic adverse reactions, as defined by EPA water quality standards criteria).			
	\	24.	Only repair, upgrades, relocations, and improvements of existing discharge pipes or replacement in-kind are allowed; no new construction of untreated discharges.			
✓		25.	Work behind cofferdams, turbidity curtains, or other instruments to control turbidity is required when operationally feasible and ESA-listed species are likely to be present (if presence is limited to rare, transient individuals, turbidity control methods are not necessary).			



HAB	HABITAT ALTERATION PDCs						
Yes	N/A	PDC#	PDC Description				
V		26.	Minimize all new waterward encroachment and permanent fill.				
	\	27.	In Atlantic salmon critical habitat, stream simulation design with a minimum span of 1.2 bankfull width will be used in areas with minimal tidal influence. In tidal areas, a design that allows for unimpeded flow will be used (no delay in water entering or exiting the area upstream of the crossing).				
	V	28.	In Atlantic salmon critical habitat, no culvert end extensions, invert line culvert rehabilitation, or slipline culvert rehabilitation may occur.				

VES	SEL TI	RAFFIC	PDCs
Yes	N/A	PDC#	PDC Description
	✓	29.	Maintain project (i.e., construction) vessels operating within the action area to speed limits below 10 knots and dredge vessels to speeds of 4 knots maximum, while dredging.
	✓	30.	Maintain a 1,500-foot buffer between project (i.e., construction) vessels and ESA-listed whales and a 300-foot buffer between project vessels and sea turtles. This also applies to dredge vessels.
	✓	31.	The number of project (construction) vessels must be limited to the greatest extent possible, as appropriate to size and scale of project.
	✓	32.	The project must not result in the permanent net increase of commercial vessels.

Justification for NLAA Determination if not Incorporating All PDC

If the project is not in compliance with all of the general and stressor-based PDCs, but you can provide justification and/or special conditions to demonstrate why the project still meets the NLAA determination and is consistent with the aggregate effects considered in the programmatic consultation, you may still certify your project through the NLAA program using this verification form. Please identify which PDCs your project does not meet (e.g., PDC 9, PDC 15, PDC 22, etc.) and provide your rationale and justification for why the project is still eligible for the verification form. Project modifications must not result in different effects not already considered.

To demonstrate that the project is still NLAA, you must explain why the effects on ESA-listed species or critical habitat are **insignificant** (i.e., too small to be meaningfully measured or detected) or **discountable** (i.e., extremely unlikely to occur). **Please use this language in your justification.**



PDC#	Justification
	一个人们是我们是这个不可以是是一种不是,我们就是这种的人们的,但是我们的一个一种,就是



FHWA/DOT Verification of Determination (To be filled out by FHWA/DOT staff only) By submitting this Verification Form, FHWA, or the state DOT as FHWA's designated non-federal representative, indicates that they determined that the proposed activity described above is not likely to adversely affect (NLAA) ESA-listed species or designated critical habitat under NMFS jurisdiction in accordance with the Program, and all effects (direct, indirect, interrelated, and interdependent) are either insignificant (so small they cannot meaningfully be measured, detected, or evaluated) or discountable (extremely unlikely to occur).

✓	In accordance with the FHWA GARFO NLAA Program, we have determined that the action complies with all applicable PDCs and is not likely to adversely affect listed species.					
	In accordance with the FHWA GARFO NLAA Program, we have determined that the action is not likely to adversely affect listed species per the justifications and/or special conditions provided above.					
	FHWA/DOT Signature: Date:					
Nico	Nicole Lineberry N. B-US, E-nicole leporacci@dot.ri.gov, O=RIDOT, OU-Environmental Division *, CN=Nicole Lineberry Out - Environmental Division *, CN=Nicole Lineberry Out - Environmental Division *, CN=Nicole Lineberry Out - Ou					

By providing your determination and signature, you are certifying that to the best of your knowledge the information provided in this form is accurate and based upon the best available scientific information. This form must be filled out and signed by FHWA or state DOT staff, as an officially designated non-federal representative.

GARFO PRD Concurrence (To be filled out by GARFO PRD)

After receiving the Verification Form, GARFO PRD will contact FHWA/DOT with any concerns and indicate whether GARFO PRD concurs with FHWA/DOT's determination.

HWA/DOT's determ								
FHWA/DOT's determination that the action complies with all applicable PD								
not likely to adversely affect listed species or critical habitat.								
n accordance with the	FHWA GARFO NLAA Program	m, GARFO PRD concurs with						
'HWA/DOT's determine	ination that the action is not like	ly to adversely affect listed						
bove.								
GARFO PRD does not concur with FHWA/DOT's determination that the action								
GARFO PRI	D Signature:	Date:						
EZ.ROOSEVELT.AN 6982881	Digitally signed by MESA GUT!ERREZ.ROOSEVELT.ANDRES 158 6982881 Date: 2024.03.13 13:41:23 -04'00'	03/13/2024						
	n accordance with the HWA/DOT's determined by the second of the second o	n accordance with the FHWA GARFO NLAA Program HWA/DOT's determination that the action is not likely pecies or critical habitat per the justifications and/or sphove. GARFO PRD does not concur with FHWA/DOT's determination with the applicable PDCs (with or without just in individual Section 7 consultation to be completed in GARFO NLAA Program. GARFO PRD Signature: Digitally signed by MESA GUTIERREZ.ROOSEVELT.ANDRES. 158 6982881						



NATIONAL MARINE FISHERIES SERVICE (NMFS) ENDANGERED SPECIES ACT (ESA)AND ESSENTIAL FISH HABITAT (EFH) PROTECTION

Essential fish habitat consultation under the Federal Highway Administration (FHWA)/NMFS Greater Atlantic Fisheries Office (GARFO) Individual Abbreviated consultation and ESA Section 7 consultation under the FHWA GARFO NLAA Program was completed for the replacement of the Barrington and Warren East Bay bike path bridges over the Barrington and Palmer River. Compliance with the requirements below is necessary to ensure compliance with the consultations:

Ensure all operators, employees, and contractors are aware of all FHWA environmental commitments when working in areas where EFH and ESA-listed species may be present.

Contact the RIDOT Natural Resources Unit (401-479-1327) and **dot.nru@dot.ri.gov** for questions about restrictions or conservation measures.

Time of Year (TOY) Restrictions (Essential Fish Habitat)

Any work that causes greater than minimal turbidity should not be conducted from Feb. 1-June 30, of any year, to protect sensitive life stages of winter flounder.

Pile Driving Restrictions

If pile driving occurs the following is required:

A "soft start" is required to allow animals an opportunity to leave the project vicinity before sound pressure levels increase. In addition to using a soft start at the beginning of the workday for pile driving, one must also be used at any time following cessation of pile driving for a period of 30 minutes or longer.

For impact pile driving: pile driving will commence with an initial set of three strikes by the hammer at 40% energy, followed by a one-minute wait period, then two subsequent three-strike sets at 40% energy, with one-minute waiting periods, before initiating continuous impact driving.

For vibratory pile installation: pile driving will be initiated for 15 seconds at reduced energy followed by a one-minute waiting period. This sequence of 15 seconds of reduced energy driving, one-minute waiting period will be repeated two additional times, followed immediately by pile-driving at full rate and energy.

Conservation Measures

Any temporary discharges must meet state water quality standards (e.g., no discharges of substances in concentrations that may cause acute or chronic adverse reactions, as defined by EPA water quality standards criteria).

Prevent construction debris and sediment from entering aquatic areas and remove all construction debris and excess/deteriorated materials and dispose of in an upland area. Ensure that raw concrete does not contact the water; wet pours of concrete must be confined within sealed forms until the concrete is set or pre-cast members installed.



Any in-water lines, ropes, or chains must be made of materials and installed in a manner to minimize or avoid the risk of entanglement by using thick, heavy, and taut lines that do not loop or entangle. Lines can be enclosed in a rigid sleeve.

Return areas impacted by temporary activities, fills, or structures to pre-construction or better condition, including elevations and substrate, and replant with native species.



This page intentionally left blank.



NOAA Fisheries GARFO HCD EFH Consultation



This page intentionally left blank.



Re: RIDOT - East Bay Bike Path - Barrington and Warren Bridges Project Modification - Appl No. 2019-EH-0221

Sabrina Pereira - NOAA Federal <sabrina.pereira@noaa.gov>

Thu 3/7/2024 4:12 PM

To:Leporacci, Nicole (DOT) < Nicole.Leporacci@dot.ri.gov>

Cc:Scott Hobson <shobson@vhb.com>;Hamilton, Heather (DOT) <Heather.Hamilton@dot.ri.gov>;Schneider, Eric (DEM) <eric.schneider@dem.ri.gov>;Akinfolarin, Hamid (DOT) <Hamid.Akinfolarin@dot.ri.gov>;Pompei, Anthony (DOT) <ahching.pompei@dot.ri.gov>;Padilla, Carlos (FHWA) <carlos.padilla@dot.gov>;D'Alessandro, Michael (FHWA) <michael.dalessandro@dot.gov>;Palumbo, Vincent (DOT) <vincent.palumbo@dot.ri.gov>

This Message Is From an External Sender

This message came from outside your organization.

Report Suspicious

Hi Nicole,

Thank you for sending this information, and again for the meeting a few weeks ago to discuss this project's updates. After reviewing the proposed updates, we have no additional conservation recommendations to provide.

Please let me know if you have any additional questions, and thank you for coordinating with us.

Best wishes,
Sabrina Pereira
Marine Habitat Resource Specialist
Habitat and Ecosystem Services Division
NOAA/ National Marine Fisheries Service
Narragansett, RI
she/her/hers
(978)-675-2178
Sabrina.pereira@noaa.gov

On Thu, Feb 29, 2024 at 1:03 PM Leporacci, Nicole (DOT) < Nicole.Leporacci@dot.ri.gov > wrote: Good afternoon Sabrina.

The Rhode Island Department of Transportation (RIDOT) submitted an individual abbreviated consultation (Appl No. 2019-EH-0221) for the proposed removal and reconstruction of the East Bay Bike Path Bridges across the Barrington River and Palmer River in Barrington and Warren, RI. Please find attached a narrative explaining the modifications to the construction of the bike path bridges, as discussed at our meeting on February 26th.

Design plans can be found at the link below:

EBBP Bridges - Volume 1 and 2 plan sets with fishing structures [vhb-my.sharepoint.com]

Please let me know if you need any more additional information. Thank you!

Best, Nicole

Nicole Lineberry (Leporacci) Senior Environmental Scientist



Natural Resources Unit, RIDOT E: nicole.leporacci@dot.ri.gov



Re: East Bay Bike Path Bridges - Barrington & Warren, RI

Sabrina Pereira - NOAA Federal <sabrina.pereira@noaa.gov>

Wed 5/10/2023 4:38 PM

To:Johnstone, Erik (DOT) <erik.johnstone@dot.ri.gov>
Cc:Breen, Daniel B CIV USARMY CENAE (US) <Daniel.B.Breen@usace.army.mil>

This Message Is From an External Sender

This message came from outside your organization.

Report Suspicious

Hi Erik,

Thanks again for setting up the call today with your team to discuss the design updates to the East Bay bike path bridges project in the Barrington and Palmer Rivers. It was great to hear that the inwater work has been greatly minimized since RIDOT completed the EFH consultation with us last February 2022, and that the creosote piles will be directly pulled and/or cut below the mudline.

Your team noted that most of the in-water work in the Barrington River will be constructing a single, central pier (as opposed to the 2 piers that were previously proposed). The work will include pile drilling (not vibratory driving) 10 micropiles that will be capped with concrete. The piles will be installed from a spudded barge, and the bridge spans will be placed from the shoreline.

In the Palmer River, some of the changes to the project include: installing supplemental riprap around the bridge abutments that will encroach slightly beyond the existing riprap footprint, and impacts to wetlands due to temporary excavation and permanent fill. Your team mentioned that RIDOT is proposing wetland restoration as mitigation for the permanent fill, and 3 years of wetland vegetation monitoring post-implementation.

Based on the information provided during today's call, it seems like the in-water work you described would cause minimal turbidity and could therefore be conducted during the winter flounder and diadromous time of year restrictions that we previously recommended in our consultation last year. Since there will be ample migratory corridor for any potential diadromous fish passage during in-water construction, I would amend our conservation recommendation 2 to the following:

CR 2: Any work that causes greater than minimal turbidity should not be conducted from Feb. 1-June 30, of any year, to protect sensitive life stages of winter flounder.

I still recommend CR 3 (soft start to pile-driving activities) just in case the contractors have to use vibratory pile driving methods instead of the currently proposed pile drilling.

I hope this helps clarify our thoughts on the currently proposed project, but please let me know if you have any additional questions or needs. If other significant changes are proposed for the project going forward, please reach out to us for discussion or to reinitiate EFH consultation.

Thank you again for coordinating with us.

Sabrina Pereira

Marine Resources Management Specialist Habitat and Ecosystem Services Division NOAA/ National Marine Fisheries Service Gloucester, MA



Pronouns: she/her/hers (978)-675-2178

Sabrina.pereira@noaa.gov

On Mon, May 1, 2023 at 10:47 AM Johnstone, Erik (DOT) < erik.johnstone@dot.ri.gov wrote: Hello again!

Thank you for your quick reply. I am checking in with Eric Schneider and VHB to nail down a time. I'll let you know as soon as I hear back.

Thanks again!

Erik A. Johnstone Principal Environmental Scientist Natural Resources Unit

From: Sabrina Pereira - NOAA Federal < sabrina.pereira@noaa.gov>

Sent: Monday, May 1, 2023 9:54 AM

To: Johnstone, Erik (DOT) < erik.johnstone@dot.ri.gov>

Subject: Re: East Bay Bike Path Bridges - Barrington & Warren, RI

Hi Erik,

Thanks for reaching out. This week I am traveling for work and therefore unavailable from 5/1 through 5/5. I have availability from 12-1 on 5/8, from 10-2 on 5/9, and 1:30-3 on 5/10. From 5/11 - 5/26 I will be away on leave.

Please let me know what works best for you and your team.

Best wishes,
Sabrina Pereira
Marine Resources Management Specialist
Habitat and Ecosystem Services Division
NOAA/ National Marine Fisheries Service
Gloucester, MA
Pronouns: she/her/hers

Pronouns: she/her/hers

(978)-675-2178

Sabrina.pereira@noaa.gov

On Mon, May 1, 2023 at 9:04 AM Johnstone, Erik (DOT) < crik.johnstone@dot.ri.gov wrote:

Hi Sabrina:

I hope that this note finds you well. I'm reaching out again regarding the East Bay Bike Path bridges in Barrington and Warren, RI. The design and permitting have been advancing, and our selected Design/Build Entity has been designing the bridges and reviewing riverbed conditions with an eye for further reducing in-water impacts and minimizing turbidity. Together our D-B Entity have been coordinating with Eric Schneider of RIDEM's Marine Fisher 3/18/2024 discuss the required in-water work activities and explore the possibility of conductions.

the TOYRs. Our recent PCN application filings with the Army Corps have introduced that possibility, and we have requested relief from the Corps' TOYRs.

We would like to respectfully request another consultation meeting with you to present the updated project designs and discuss the proposed work activities with respect to fisheries protection. The meeting would reflect that only one bridge pier is now required for each bridge and that much of the work will occur within the intertidal zones and within riverbed areas exhibiting coarse bottom substrates. We expect that meeting attendees would include Eric, design and permitting representatives of the D-B Entity, myself, and other RIDOT representatives.

Eric Schneider has indicated that the timeframes of May 1 - 5 and 10-12 would work best with his schedule. Please feel free to provide meeting dates and times that would work best for you.

Thank you once again.

Erik

Erik A. Johnstone

Principal Environmental Scientist

Natural Resources Unit

Rhode Island Department of Transportation



Federal Interagency Comment Form

Date: 02/18/2022

Project: East Bay Bike Path Bridge Replacements (Barrington and Warren, RI)

Appl No.: 2019-EH-0221

Commenting Agency: NOAA/NMFS/GARFO/HCD

Action Agency Project Manager: Nicole Lineberry, RIDOT

Waterway: Barrington River (Barrington bridge) and Palmer River (Warren bridge) in Rhode Island.

Activity: Replacing 2 bridges on the East Bay Bike Path. There will be 1,702 SF of impacts at the Barrington bridge location, and 1,516 SF of impacts at the Warren bridge location. The existing bridges will be completely demolished and removed, with use of a S-50 series Quadrafloat work barge with spuds or similar. Existing timber supports will be cut off two feet below the mud line and removed since they were previously treated with creosote. The new bridges will approximate the same deck dimensions (300 ft long) but will be supported by only two intermediate sets of stainless steel micropiles, both set 100 feet from each bank, allowing three horizontal clearance gaps of 100 feet each.

ESSENTIAL FISH HABITAT (EFH)

Project may adversely affect EFH. Areas are designated EFH for 16 federally managed species, including winter flounder, and HAPC for summer flounder and juvenile cod. There is no SAV or substantial macroalgae present at either project site.

ESSENTIAL FISH HABITAT CONSERVATION RECOMMENDATIONS: (Note: EFH CRs require a response from the federal action agency within 30 days of receipt or 10 days before a permit is issued if CRs are not included as a special condition of the permit. In addition, a distinct and further EFH consultation must be reinitiated pursuant to 50 CFR 600.920 (j) if new information becomes available, or if the project is revised in such a manner that affects the basis for the above EFH determination or EFH conservation recommendations.)

- 1. Creosote-treated piles should be cut 2 feet below the mudline if they cannot be removed.
- 2. No in water work should occur from February 2 June 30 to protect sensitive winter flounder life stages, and migrating diadromous fish.
- 3. Use a soft start each day of pile driving, after a break of 30 minutes or more, and if any increase in pile installation or removal intensity is required. Build up power slowly from a low energy start-up over a 20-minute period to warn fish to leave the vicinity. This buildup shall occur in uniform stages to provide a constant increase in output.

FISH AND WILDLIFE COORDINATION ACT COMMENTS ENDANGERED SPECIES

Threatened or endangered species under the jurisdiction of NMFS may be present in the project area. The federal action agency will be responsible for determining whether the proposed action may affect listed species. If they determine that the proposed action may affect a listed species, they should submit their determination of effects, along with justification and a request for concurrence to the attention of the Section 7 Coordinator, NMFS, Greater Atlantic Regional Fisheries Office, Protected Resources Division, 55 Great Republic Drive, Gloucester, MA 01930 or nmfs.gar.esa.section7@noaa.gov. If you have any questions regarding these comments, please contact Roosevelt Mesa at Roosevelt.Mesa@noaa.gov.

OTHER: Provide a copy of the permit when issued.	
Prepared by: _Sabrina Pereira	date:02/18/2022



February 7, 2024

Ref: 73305.01

Mr. Jeffrey Willis, Executive Director Coastal Resources Management Council Oliver H. Stedman Government Center 4808 Tower Hill Road Wakefield, RI 02879-1900



Re: Design Modifications – Fishing Accommodations and Riprap Scour Protection at Center Piers RIDOT – East Bay Bike Path Bridge Replacements – Reconstruction Barrington River Bridge (RIDOT Bridge No. 083751) and Palmer River Bridge (RIDOT Bridge No. 083851), Barrington and Warren, RI PTSID No. 0881A CRMC Application No. 2023-04-094

Dear Mr. Willis:

The Rhode Island Department of Transportation (RIDOT) submitted a Category B Assent Application on April 19, 2023 for proposed reconstruction of the East Bay Bike Path Bridges across the Barrington and Palmer Rivers in Barrington and Warren (the Project). A public hearing for the Project was held before the full Council on October 24, 2023, at which time the Council requested that public fishing accommodations be incorporated into the Project design. Additionally, final design calculations have been conducted relative to the single, central pier of each bridge, and RIDOT finds it prudent to propose riprap scour protection at both piers. The Project drawings submitted initially with the Category B application proposed scour protection at each bridge abutment but did not include it at the center piers.

Fishing Accommodations

In accordance with the Council's request, RIDOT has analyzed alternatives for providing fishing accommodations at each bridge location, including widening of the proposed Bike Path bridges. The Applicant's design team strove to minimize the extent of fills proposed in tidal waters in the initial plan submission – using measures such as permanent retaining walls, lowered abutment and wall footing elevations, narrowed temporary bridge launching pads, and temporary construction walls. Additional widening would require a substantial volume of additional fills in tidal waters, particularly given the increased bridge heights, and would introduce fills to coastal wetlands previously avoided. The added weight and width of widened bridges would jeopardize the ability to implement the launching alternative that has been designed specifically to avoid conflict with overhead electric and communication wires and poles. The added weight of a widened bridge would necessitate an additional pier support in the river, and preliminary cost estimates to widen the bridges would far exceed the available project budget. In light of these considerations and constraints, RIDOT is proposing independent public fishing structures that would not jeopardize the bridge designs and loadings. RIDOT anticipates that each fishing structure would provide casting reach to the deepest waters within each channel and would enable fishing in both tide swings.

Barrington River Fishing Accommodation:

A new pile-supported, timber fishing pier is proposed on the east side of the Barrington River, north of, and parallel with, the proposed bridge. The elevation of its proposed terminal deck is set lower than the proposed bike path, yet above the 100-year flood elevation, and its accessway way will transition from the Bike Path pavement to the terminal deck at an accessible grade. Structural support for the fishing pier will be accomplished with drilled and socketed steel pipe piles. Consideration was given to driven piles, but drilling was deemed more effective due to the presence of shallow bedrock, and the potential for concussive implications to marine life will be avoided.

Palmer River Fishing Accommodation:

The temporary bulkhead installed to accommodate construction barge loading and offloading on the west shoreline of the Palmer River has been identified by RIDOT as an appropriate accommodation for public fishing, and RIDOT has observed persons fishing from the structure. RIDOT proposes to retain and improve the temporary bulkhead to serve as a permanent public fixture. The bulkhead was designed, permitted, and installed with no

Mr. Jeffrey Willis, Executive Director Ref: 73305.01 February 7, 2024

Page 2



impact to coastal wetlands or to other special aquatic sites. An accessible paved walkway will be constructed to the bulkhead terminus, and a concrete cap and timber railing will be installed along the top of the sheet piling. Additional riprap armor is proposed along the walkway's north side to provide long term erosion protection and to accommodate a slightly heightened surface elevation, and protective riprap will be installed along the sheet piling face. The land area between the fishing walkway and Bike Path will be regraded, loamed, and seeded and will incorporate a subtle, shallow swale graded to drain east. The initial restoration landscape planting plan has been modified for the area between the two paths, using the same native woody species proposed previously.

Riprap Scour Protection at Bridge Piers

Final design calculations of local riverbed scour and of excessively large storm events indicate that scour protection is warranted to prevent undue deflection in the piles comprising the center pier at each bridge location. Calculations indicate that protective riprap should be installed at a minimum depth of six feet for a minimum distance of eight feet from each pile, forming a level shelf. The footprint and volume of stone required, in consideration of the topographical characteristics of each river, are reflected on the Project drawings and summarized in attached Tables 4-1 and 4-2. Riprap is proposed as R-8, or D-50, with 24-inch minimum diameter, based on engineering calculations (see pages 6 and 7 of the attached riprap sizing calculations stamped by a RI-registered professional engineer). A base layer of sand filled geotextile containers is proposed to be placed on the riverbed to stabilize the scour footprint around the piles. Riprap is specified to be installed on top of the geotextile containers through a combination of dumping and careful placement to ensure that the pile casings and geotextile containers are not damaged. Given its mounded configuration above the riverbed surface, the stone may serve an underwater benefit for marine life, potentially providing refuge and down-current protection from strong tides.

Please find enclosed four copies of the following materials: this transmittal letter, revised Project plan sets for Volumes 1 and 2, stamped riprap sizing calculations, and revised Tables 4-1 and 4-2 identifying updated areas and volumes for the Project's proposed fills in tidal waters. Plans, profiles, and cross sections related to the fishing structures and bridge pier riprap have been outlined with revision clouds on the revised Project plans for ease of identification. Please note that RIDOT will provide the same information to the RI Department of Environmental Management for the State Water Quality Certification applications currently under review and is additionally coordinating with the US Army Corps of Engineers.

Thank you, and please feel free to contact either Ms. Alisa Diaz Richardson of RIDOT at (401) 479-1327 or Alisa Richardson@dot.ri.gov, or me at (401) 457-7824 or shobson@vhb.com, if you have any questions or require additional information.

Sincerely,

Scott S. Hobson, PWS Senior Ecologist

Son S. Holm

cc: Alisa Diaz Richardson, MS, PE, PMP, RIDOT Hamid Akinfolarin, Project Manager I, RIDOT Andrew F. Prezioso, PE, VHB Andres Aveledo, Project Manager, Aetna Bridge

FEB 0 7 2024

COASTAL PERUMICAS.

MANAGEMENT COUNTY

Appendix 5: Riprap Sizing Calculations





The second secon					
	Project:	EBBP - Barrington & Palmer River Crossings	Project #	73305.01	- Tank
	Location:	Barrington and Warren, RI	Sheet		
	Calculated by:	DWC	Date:	4/4/2023	
	Checked by:	AP	Date:	4/5/2023	
	Title:	Bridge 083751 and 083851: Riprap Sizing at A	tions		
				AND DESCRIPTION OF THE PERSON NAMED IN COLUMN 1	

Calculation Goal:

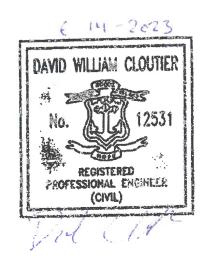
Calculate appropriate riprap stone sizing for scour and wave protection at bridge abutments

Assumptions and Sources;

Hydraulic input data from 2014 Design Study Report (DSR) for Bridges No. 083701 and 083801, October 2014
HEC-RAS hydraulic model results from 2014 NorthStar Hydro model prepared for 2014 DSR
All elevations are referenced to NGVD29, matching Design Study Report and HEC-RAS model
Storm surge and wave data from National Flood Insurance Program (NFIP) Flood Insurance Study (FIS),
Bristol County, Rhode Island (FIS No. 44001CV000B), Effective Date July 7, 2014
Riprap design guidance from Federal Highway Administration (FHWA) Hydraulic Engineering Circular
(HEC) 23, Bridge Scour and Stream Instability Countermeasures: Experience, Detection and Design, September 2009:
Design Guide 14; Rock Riprap at Bridge Abutments
Design Guide 17; Riprap Design for Wave Attack

Riprap sized for coastal storm surge 1% annual exceedance probability (1% AEP, a.k.a. "100-year") event DSR provides peak flood elevations and peak flood velocities in summary tables; however, these conditions do not occur simultaneously. Therefore, riprap calculations use values directly from HEC-RAS hydraulic model outputs. Due to constantly varying water surface elevation, velocity, and flow rates during coastal storm surge events, riprap sized for hydraulic conditions at the time of peak flow through the crossings from HEC-RAS hydraulic model Peak flow occurs between HEC-RAS hydraulic model time Jan 2 15:00 and 15:30; average values used for this period Peak flow is negative, indicating flood storm surge travelling upstream; therefore HEC-RAS hydraulic model data at the south ("downstream") fascia of bridge structures used for riprap design.

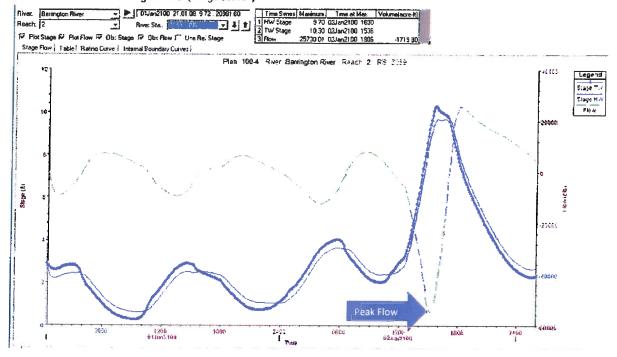
Riprap sizing recommendations are based on RIDOT Standard Specifications "Blue Book," Section M10 (2022 Edition)



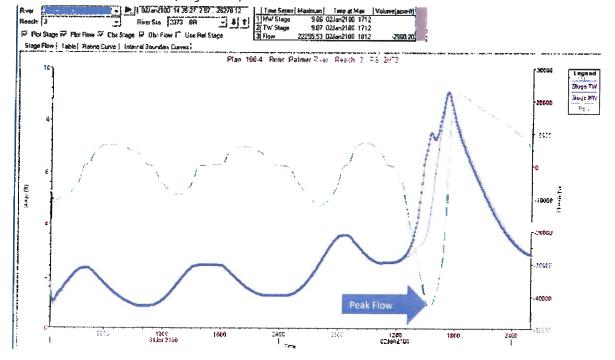


Appendix S: Riprap Sizing Calculations

HEC-RAS Model Results: Barrington River (Bridge 083751)



HEC-RAS Model Results: Palmer River (Bridge 083851)



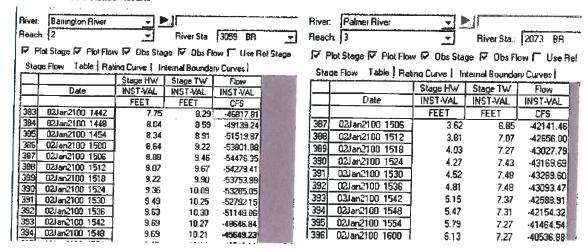
RECEIVED

FEB 0 7 2024 Page 2 of 7

COASTAL RESOURCES MANAGEMENT COUNCIL

Appendix S: Riprap Sizing Calculations

HEC-RAS Model: Detailed Results



River	River Sta	Profile	W.S. Elev.	Q Total	Area	Vel. Total	Hydr. Depth	Froude #
			(ft)	(cfs)	(sq ft)	(ft/s)	(ft)	
Barrington Riv	er 3021	02JAN2100 1500	9.22	-53801.8	10157.51	-7.84	22.43	0.31
Barrington Riv	rer 3021	02JAN2100 1506	9.46	-54476.3	10326.18	-7.86	22.66	0.3
Barrington Riv	rer 3021	02JAN2100 1512	9.67	-54279.4	10482.29	-7.76	22.87	0.3
Barrington Riv	er 3021	02JAN2100 1518	9.9	-53753.9	10647.92	-7.6	23.1	0.29
Barrington Riv	er 3021	02JAN2100 1524	10.09	-53285	10789.72	-5.26	17.08	0.28
Barrington Riv	er 3021	02JAN2100 1530	10.25	-52792,1	10906.09	-5.16	17.2	0,28
ţ	JSE AVERAGE FOR SCI	DUR CALCULATION:	9.8	53731.4	10551.6	6.9	20.9	0.29
Palmer River	2060	02JAN2100 1500	5.57	-40888.3	3508.58	-11.65	12.85	0.57
Palmer River	2060	02JAN2100 1506	6.85	-42141.4	3556.31	-11.85	12.98	0.58
Palmer River	2060	02JAN2100 1512	7.07	-42656	3616.76	-11.79	13.15	0.57
Palmer River	2060	02JAN2100 1518	7.27	-43027.7	3672.97	-11.72	13.36	0.56
Palmer River	2060	02JAN2100 1524	7.43	-43169.6	3715.74	-11.62	13.51	0.56
Palmer River	2060	02JAN2100 1530	7.48	-43289.6	3731.41	-11.5	13.57	0.56
U	ISE AVERAGE FOR SCO	OUR CALCULATION:	7.1	42528.8	3633.6	11.7	13.2	0.57



Appendix 5: Riprap Sizing Calculations

Riprap Sizing Worksheet



Project:	EBBP - Barrington & Palmer River Cros Project #		73305.01
Location:	Barrington and Warren, RI	Sheet	2
Calculated by	DWC	Date:	4/4/2023
Checked by:	Checked by: AP		4/5/2023
Title:	Bridge 083751 and 083851: Riprap Sizing	at Abutments	, HEC-23 DG 14

Notes:

- 1) Calculations based on methodology outlined in HEC-23 3rd Edition (FHWA-NHI-09-112, 2009), Design Guide 14
- 2) Scour Countermeasure Design Storm = 100 year (coastal storm surge)
- 3) Hydraulic inputs from HEC-RAS model provided by RIDOT from 2014 Bridge Design Study Report

A) Determine Set-Back Ratio (SBR)

Bridge No.	83751	83851	
River	Barrington River	Palmer River	
Setback Length	0	0	ft
Avg. Chan, Flow Depth	20.89	13.24	ft
SBR	0	0	7

SBR < 5: V based on entire contracted area through bridge

B) Determine Minimum Riprap Size At Abutments (Eq. 14.1 or 14.2)

For Fr < 0.80: (Eq 14.1) where:

D ₅₀	<u> </u>	V ₂	Den
	=		· ·
У	$(S_{-} - 1)$	QV I	¥

= median stone diameter, ft (m)

 characteristic average velocity in the contracted section (explained below), ft/s (m/s)

S_s = specific gravity of rock riprap For Fr >=0.80: (Eq 14.2)

g = gravitational acceleration, 32.2 ft/s² (9.81 m/s²)
y = depth of flow in the contracted bridge opening, ft (m)
K = 0.89 for a spill-through abutment

1.02 for a vertical wall abutment

Q	53,731.42	42,528.77	cfs	Flow Through Bridge Opening
Α	10,551.62	3,633.63	sf	Contracted Flow Area At Bridge
٧	6.91	11.71	ft/s	Average Flow Velocity At Bridge
Sg	2.65	2.65	pcf	Specific Gravity of Rip Rap
g	32.20	32.20	ft/s2	Gravitational Acceleration
у	20.89	13.24	ft	Average Channel Depth At Bridge
K	0.89	0.89	ft	Spill-Through Abutment, Fr< 80
Fr	0.29	0.57	1	Froude Number

D ₅₀	8.0	2.3	median stone diameter, ft
Dso	9.6	27.5	median stone diameter, inche

Note: For Palmer River, R-7 size riprap acceptable due to documented stability of existing riprap in light of major storm history and D $_{100}$ size greater than calculated required riprap size for stability.

C) Determine Recommended Riprap Extents

Flow Depth	20.89	13.24	ft	
Extent from Toe	25	25	ft	Note: if existing riprap is in place, stop before 25-ft extent
Extent Downstream	25	25	ft	

RECOMMENDATION: USE R-7 SIZE RIPRAP (D $_{50}$ =18" ,D $_{100}$ =34") AT BOTH LOCATIONS

Recommended D ₅₀	1.5	1.5	ft
Minimum Riprap Thickness	3.0	3.0	ft



Appendix S: Pincan Sizing Calculations

Coastal Scour Countermeasures Sizing Worksheet



RIPRAP
REVETMENT
SIZING

THE RESERVED			
Project:	EBBP - Barrington & Palmer River Crossi	Project #	73305.01
Location:	Barrington and Warren, RI	Sheet	3
Calculated by:	DWC	Date:	4/4/2023
Checked by:	AP	Date:	4/5/2023
Title:	Bridge 083751 and 083851: Riprap Sizing fo	r Wave Attack, HEC-23 DG 17	

Notes:

- 1) Calculations based on methodology outlined in HEC-23 3rd Edition (FHWA-NHI-09-112, 2009), Design Guide 17
- 2) Scour Countermeasure Design Storm = 100 year (coastal storm surge)
- 3) Hydraulic inputs from 2014 National Flood Insurance Program (NFIP) Flood Insurance Study (FIS)

Assumptions:

- 1. Use Hudson's Equation for riprap size (HEC-23 Eqn 17.8)
- 2. Assume specific gravity of stone $S_1 = 2.65$, unit weight of stone $w_1 = 165$ lb./ft $\frac{3}{2}$, and empirical coefficient $K_d = 2.2$
- 3. Assume design wave heights from FEMA FIS Coastal Transects 9/10: H=2.0 ft
- 4. Assume 1.5:1 riprop slope (0 = 33.7°)

TABLE 4 - TRANSECT DESCRIPTIONS

ELEVATION (feet NAVD881)

TRANSECT		LOCATION The transect is located at the	I-PERCENT- ANNUAL-CHANCE STILLWATER!	MAXIMUM 1-PERCENT-ANNUAL- CHANCE WAVE CREST ²			
9		The transect is located at the intersection of Union Street and Bowen Street, extending north towards the Palmer River.	₹.6	13.6			
10		The transect is located at the intersection of State Street and loyce Street extending southwest owards the Warren River.	11.4	13.4			
$W_{50} = \frac{\gamma H^3}{K_d(S_r)}$	(tar - S	$\left(\frac{\partial \theta}{\partial w}\right)^3$		(17.8)			
where:							
W_{50}	-	Weight of the median riprap part	ticle size, Ib (kg)				
γ.	=	Unit weight of riprap, lb/ft3 (kg/m	3)				
Н	=	Design wave height, ft (m)					
		(Note: Minimum recommended value h wave, $H_{0.10} = 1.27H_s$)	or use with the Hudson equat	ion is the 10 percent			
K _d	æ	Empirical coefficient equal to 2.2	for riprap				
S,	=	Specific gravity of riprap					
S _w	=	Specific gravity of water					
		(1.0 for fresh water, 1.03 for sea	water)				
θ	=	Angle of slope inclination					

Palmer River

Barrington River

H (ft)	θ (deg)	tan θ	W _{so} (lbs.)	D _{so} (in)
2.0	33.7	0.67	246.8	17.0
2.0	33.7	0.67	246.8	17.0

RECOMMENDATION: USE R-7 SIZE RIPRAP (D $_{50}$ =18" ,D $_{100}$ =34") AT BOTH LOCATIONS

Recommended D_{SD} 1.5 ft Minimum Riprap Thickness 3.0 3.0 ft

73305.01 E88P Barrington-Warren

RECEIVED

FEB 0 7 2024

COASTAL Noods, Page 5 of 7

Appendix S: Riprap Sizing Calculations





Project:	EBBP - Barrington & Palmer River Cros	Project #	73305.01
Location:	Barrington and Warren, RI	Sheet	4
Calculated b	y DWC	Date:	6/14/2023
Checked by:	NDR	Date:	6/14/2023
Title:	Bridge 083751 and 083851: Riprap Sizing	at Bridge Pie	rs, HEC-23 DG 11

Notes:

- 1) Calculations based on methodology outlined in HEC-23 3rd Edition (FHWA-NHI-09-112, 2009), Design Guide 11
- 2) Scour Countermeasure Design Storm = 100 year (coastal storm surge)
- 3) Hydraulic inputs from HEC-RAS model provided by RIDOT from 2014 8ridge Design Study Report

A) Determine Design Velocity at Pier, V_{des} (Eqns. 11.2 and 11.3)

$$V_{des} = K_1 K_2 V_{avg}$$
 (11.2)

$$V_{des} = K_1 V_{max}$$
 (11.3)

where: V_{des} = Local velocity at pier, ft/s (m/s)

K. = Shape factor equal to 1.5 for round-nose piers or 1.7 for square-faced

piers

K₂ = Velocity adjustment factor for location in the channel (ranges from 0.9 for a pier near the bank in a straight reach, to 1.7 for a pier located in

for a pier near the bank in a straight reach, to 1.7 for a pier located in the main current of flow around a sharp bend)

V_{avg} = Channel average velocity at the bridge, ft/s (m/s) V_{max} = Maximum velocity in the active channel, ft/s (m/s)

Bridge No.	83751	83851	
River	Barrington	Palmer	
Kivei	River	River	
V_{avg}	6.91	11.71	ft/s
K ₁	1.5	1.5	7
K ₂	1.00	1.00	7

10.37

Average Flow Velocity through bridge (from HEC-RAS model)

Round-Nose Pier, use 1.5

Straight, wide, uniform channel - use 1.0

Design flow

B) Determine Minimum Riprap Size At Pier (Eq. 11.1)

$$d_{50} = \frac{0.692(V_{\text{des}})^2}{(S_q - 1)2g}$$
 (11.1)

where: d₅₀ = Particle size for which 50% is finer by weight, ft (m)

17.56

V_{des} = Design velocity for local conditions at the pier, ft/s (m/s)

 S_g = Specific gravity of riprap (usually taken as 2.65) g = Acceleration due to gravity, 32.2 ft/s² (9.81 m/s²)

Bridge No.	83751	83851		
River	Barrington	Palmer		
NIVEL	River	River		
V_{des}	10.37	17.56	cfs	Design flow (calculated above)
Sg	2.65	2.65	pcf	Specific Gravity of Rip Rap
g	32,20	32.20	ft/s2	Gravitational Acceleration



Appendix S: Riprap Sizing Calculations

Dsc	0.7	2.0	median stone diameter, ft
D ₅₀	8.4	24.1	median stone diameter, inches

Note: For Barrington River, use R-6 (13") size riprap: for Palmer River, use R-8 (24") size riprap. May use R-8 (24") size riprap at both bridges if desired.

C) Determine Recommended Ri	prap Exter	nts.		
Pier Diameter	3.00	3.00	ft	Combined pile group (2x 18") pier diameter
Minimum Extent from Piers	6	6	ft	Minimum distance to extend from pier piles
RECOMMENDATION: USE R-6:	SIZE RIPR	AP (D ₅₀ =13")	OR	LARGER AT BARRINGTON RIVER, R-8 (D ₅₀ =24") AT PALMER RIVER
Recommended D ₅₀	1.1	2.0	ft	, ,,
Minimum Riprap Thickness	6.0	6.0	ft	3x D50 or predicted contraction scour, whichever is greater
Extent From Piles	6	6	ft	Extend beyond predicted scour hole extent (±5 ft)

Graded Riprap Stone

National Stone Association	Size (inches) (square openings)					
Modified NSA No	100% Passing	0 - 50% Passing	0 - 15% Passing			
R-1	2	4	No.4			
R-2	4	2	1			
R-3	8	4	2			
R-4	14	7	4			
R-5	20	10	6			
R-6	26	13	8			
R-7	34	19	14			
R-8	50	24	18			

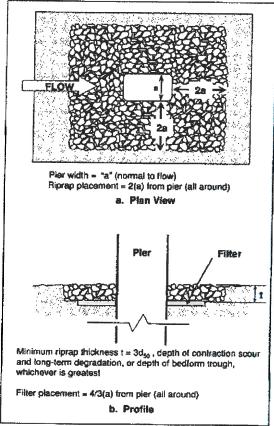


Figure 11.15. Riprap layout diagram for pier scour protection.



4.9 Summary of Project Effects in Tidal Waters and Coastal Wetlands

Below are tables provided with the initial application filings for the reconstruction of both Bike Path bridges, accompanied by the same tables revised specific to the proposed fishing accommodations and bridge pier riprap at the Barrington and Palmer Rivers. **Table 4-1** pertains to proposed fills below the high tide line (HTL) under Section 404 of the Clean Water Act, and **Table 4-2** pertains to proposed fills below mean high tide (MHT) under State Water Quality regulations and Section 10 of the Rivers and Harbors Act. **Table 4-3**, pertaining to Project effects in coastal wetlands, has not changed.

Table 4-1 (Original - April 19, 2023)

Summary of Project Effects in Tidal Waters of the Barrington and Palmer Rivers Under Section 404 (for Demolition and Reconstruction)

Location	Purpose	Temporary Fill Area* (sf)	Temporary Fill Volume * (cy)	Permanent Fill (sf)	Permanent Fill (cy)	Obstruction Removal [†] (sf)
Bridge Demol	ition (Applied for Under	Previous Catego	ory A Stage of A	pplications)		
Barrington River	Temporary Bulkhead Installation	±583	±96	0	0	
Barrington River	Bridge Pile Removal	0	0	o	0	±1,662
Palmer River	Temporary Bulkhead Installation	±2,858	±451	0	o	
Palmer River	Bridge Pile Removal	0	0	0	0	±1,420
Bridge Recons	truction (Current Stage o	of Applications)				
Barrington River	Riprap Scour Protection (at both abutments) and shoreline stabilization at east approach	0	0	±3,877	±372	~~
Palmer River	Riprap Scour Protection (at both abutments)	0	0	±3,491	±263	
Project Totals		±3,441	±547	±7,368	±635	±3,082

Calculated from the highest astronomical tide (HAT), listed by NOAA as elevation 3.78 from tide data obtained at the Providence buoy (Station 8454000), using NAVD88 datum. HAT = Section 404 HTL.



Figures obtained from the Categorical Exclusion Narrative prepared by Others, signed by FHWA on October 24, 2022.

Table 4-1 (Revised – February 1, 2024)

Summary of Project Effects in Tidal Waters of the Barrington and Palmer Rivers Under Section 404 (for Bridge Reconstruction)

Location	Purpose	Temporary Fill Area* (sf)	Temporary Fill Volume * (cy)	Permanent Fill (sf)	Permanent Fill (cy)	Obstruction Removal (sf)
Bridge Recons	struction					
Barrington River	Riprap Scour Protection (at both abutments) and shoreline stabilization at east approach	0	0	±3,877	±372	
Barrington River	Bridge Pier Riprap	0	0	±4,080	±478	**
Palmer River	Riprap Scour Protection (at both abutments)	0	o	±3,491	±263	
Palmer River	Bridge Pier Riprap	0	0	±1,522	±220	
Palmer River	Retention and Modification of Bulkhead for Fishing	0	0	±4,487	±750	
Project Totals		0	0	±17,457	±2,083	

^{*} Calculated from the highest astronomical tide (HAT), listed by NOAA as elevation 3.78 from tide data obtained at the Providence buoy (Station 8454000), using NAVD88 datum, HAT = Section 404 HTL.



Table 4-2 (Original - April 19, 2023)

Summary of Project Effects in Tidal Waters of the Barrington and Palmer Rivers – for State Water Quality Certification and Section 10 (for Demolition and Reconstruction)

Location	Purpose	Temporary Fill Area* (sf)	Temporary Fill Volume * (cy)	Permanent Fill (sf)	Permanent Fill (cy)	Obstruction Removal† (sf)
Bridge Demoi	ition (Applied for Under	Previous Catego	ory A Stage of A	pplications)		
Barrington River	Temporary Bulkhead Installation	±454	±89	0	0	
Barrington River	Bridge Pile Removal	0	0	o	0	±1,662
Palmer River	Temporary Bulkhead Installation	±2,391	±402	o	0	
Palmer River	Bridge Pile Removal	0	0	0	0	±1,420
Bridge Recons	truction (Current Stage o	of Applications)				
Barrington River	Riprap Scour Protection (at both abutments) and shoreline stabilization at east approach	0	0	±3,403	±320	
Palmer River	Riprap Scour Protection (at both abutments)	0	0	±2,098	±189	
Project Totals		±2,845	±491	±5,501	±509	±3,082

Calculated from mean high water (MHW), listed by NOAA as elevation 2.12 from tide data obtained at the Providence buoy (Station 8454000), using NAVD88 datum. MHW = Section 10 MHT.



Figures obtained from the Categorical Exclusion Narrative prepared by Others, signed by FHWA on October 24, 2022.

Table 4-2 (Revised – February 1, 2024)

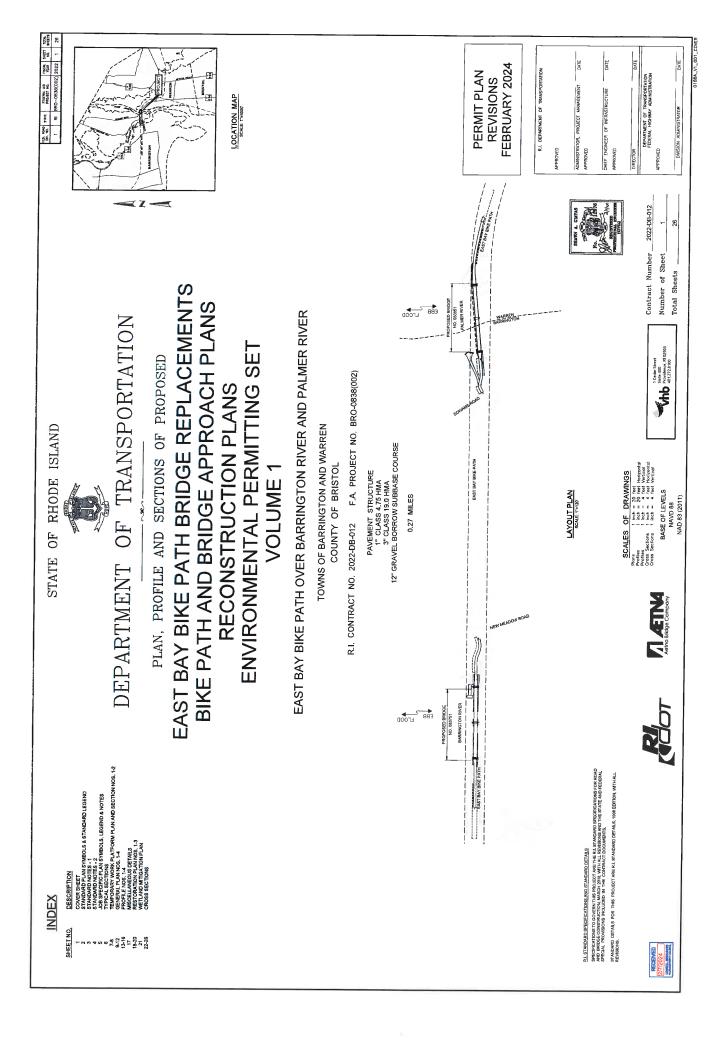
Summary of Project Effects in Tidal Waters of the Barrington and Palmer Rivers – for State Water Quality Certification and Section 10 (for Bridge Reconstruction)

Location	Purpose	Temporary Fill Area* (sf)	Temporary Fill Volume * (cy)	Permanent Fill (sf)	Permanent Fill (cy)	Obstruction Removal (sf)
Bridge Recons	truction (Current Stage	of Applications)				
Barrington River	Riprap Scour Protection (at both abutments) and shoreline stabilization at east approach	0	0	±3,403	±320	
Barrington River	Bridge Pier Riprap	0	0	±4,080	±478	••
Palmer River	Riprap Scour Protection (at both abutments)	0	0	±2,098	± 189	
Palmer River	Bridge Pier Riprap	0	0	±1,552	±220	**
Palmer River	Retention and Modification of Bulkhead for Fishing	0	0	±5,150	±668	
Project Totals		0	0	±16,283	±1,875	

Calculated from mean high water (MHW), listed by NOAA as elevation 2.12 from tide data obtained at the Providence buoy (Station 8454000), using NAVD88 datum. MHW = Section 10 MHT.

REGEIVED
FEB 0 7 2024

Revised February 1, 2024



and the	_	2022 2 28		NOT IN THIS CONSTRUCTION CONTRACT	FURNISH AND INSTALL NEW WATER GATE VALVE BOX	FURNISH AND INSTALL NEW WATER GATE VALVE AND BOX	FURNISH AND INSTALL NEW WATER CURB STOP BOX	FURNISH AND INSTALL NEW WATER CLIPP STOP AND BOX	DEBUANENT CHECK DAM		RECONSTRUCT TYPE IN CATCH BASIN TO CATCH BASIN	WITH GUTTER INLET	R.I.D.O.T. COMMUNICATIONS MANHOLE	REMOVE, HANDLE, HAUL, TRIM, RESET CURB	RELOCATE LAMP POST	TOTAL DIEGO TOTAL	RELOCATE MAILBOX (BY OTHERS)	REMOVE PAVEMENT MARKINGS	RIP-RAP PAD (SEE DETAL)	THE PERSON NAMED OF THE PE	NEWOYE AND RELUCALE SIGN	RELOCATE UTILITY POLE (BY OTHERS)	STONE BAFFLE	STEEL BEAM BRIDGE CONNECTION APPROACH END (W/O NESTED RAIL)	STEEL BEAM BRIDGE COMMECTION TRAILING FMD (WANGSTED BALL)	CTDICTION OF STREET STREET STREET STREET STREET STREET	SINUCIUNAL DISPOSITION - SEE CS PAGES OF SPECIFICATION	REMOVE AND STOCKPILE FENCE	SPECIAL GRADED AGGREGATE	REMOVE AND STOCKPILE GRANITE CLIRB		KEMOVE AND STOCKPILE GUARDRAIL	REMOVE AND STOCKPILE HYDRANT	REMOVE AND STOCKPILE SIGN	REMOVE AND STOCKBLE TRACEIC SIGNAL SYSTEM	THE STORY OF STORY STORY	CONCRETE THRUST BLOCK	TIE EXISTING PIPE INTO NEW STRUCTURE	TIE NEW PIPE INTO EXISTING STRUCTURE	THRE BEAM TRANSFRON	THREE BEAN REINGS CONNECTION	TOTAL TOTAL OF THE	Comment and worm In	4 WOOD CHIP MOLCH	4" EPOXY RESIN PAVEMENT MARKINGS - DOUBLE YELLOW	6" EPOXY RESIN PAVEMENT MARKINGS - WHITE	12" EPOXY RESIN PAVEMENT MARKINGS - WHITE	6" PREFORMED PATTERNED MARKING (HIGH PERFORMANCE TAPE)	4" EPOXY RESIN PAVEMENT MARKINGS - YELLOW	6" EPOXY RESIN PAVEMENT MARKINGS - YELLOW	PROFILE GRADE LINE																									EAST BAY BIKE PATH BRIDGE REPLACEMENT	(BARRINGTON RIVER AND WARREN BRIDGES)	ONWARREN VOLUME 1	STANDARD PLAN SYMBOLS &	STANDARD LEGEND	0188A_V1_002_STDS7N	
			((≩	BAR I	BWWN	NWCB	NWSB	()(8)(SG.	(E))(GMB)	KPW.	RRP) (E)(ES)	SBAE		(9)(b	(S)	SS)(<u>)</u> (E)	(s)	(SE)(<u>)</u> ()(-)(1		₽)(3	12W		(\$)	(2)	P,G.L.																									EAS		BARRING))			
COLEME CONCERTS CONTRACTOR	DETECTABLE WARNING STOTEM	TREE PROTECTION DEVICE	DRIP LINE TREE PROTECTION DEVICE	FOR EXISTING TREES	SHRUB PROTECTION DEVICE	TREE WELL	TREE WALL	ADJUST CATCH BASIN TO GRADE	ADJUST CATCH BASIN TO MANHOLE	ADJUST CHIEB STOP TO CRADE	APRILIST DESIRANDE MANUAL OF TOTAL	ACCOST DRAINAGE MANNOCE TO GRADE	ADJUST ELECTRIC MANHOLE TO GRADE	ADJUST FRAME AND COVER TO GRADE	ADJUST FRAME AND GRATE TO GRADE	AN HIGH CAS CASE SAN IN THE PARTY AND INC.	ALJUST GAS GATE BOX TO GRADE	ADJUST HANDHOLE TO GRADE	ADJUST SANITARY SEWER MANHOLE TO GRADE	ADJUST TELEPHONE MANHOLF TO CRADE	ADDIEST WATER CATE BOX TO CRADE	AUTOSI WAIER CAIE BOX IO GRADE	3 CLASS 9.5 HMA	B" GRAVEL BORROW SUBBASE COURSE	BUILD NEW STRUCTURE OVER EXISTING PIPE	CLEAN CATCH BASIN	The state of the s	CUI AND CAP PIPE WITH RESTRAINT (ALL SIZES)	CLEAN AND FLUSH PIPE	CLEARING AND GRUBBING	O FAN MANHOLE	The second second	Authing cold Plane	CUT AND PLUG PIPE (ALL TYPES, ALL SIZES)	REMOVE AND DISPOSE BITUMINOUS CURB	BELLOUE AND DISBOSE CONCENTER TO	THE PROPERTY OF THE PROPERTY O	KEMOVE AND DISPOSE CATCH BASIN	REMOVE AND DISPOSE DROP INLET	REMOVE AND DISPOSE FENCE	REMOVE AND DISPOSE FRAME AND COVER	REMOVE AND DISPOSE FLARED END SECTION	REMOVE AND DISPOSE FRAME AND CRATE	DESCRIPTION OF STREET OF STREET	NEMOVE AND DISPOSE FIRE HYDRANT	REMOVE AND DISPOSE FLEXIBLE PAVEMENT	REMOVE AND DISPOSE GUARDRAIL	REMOVE AND DISPOSE HEADWALL	REMOVE AND DISPOSE HIGHWAY BOUND	REMOVE AND DISPOSE HANDHOLE	REMOVE AND DISPOSE LIGHT AND FOUNDATION	REMOVE AND DISPOSE MEDIAN BARRIER	REMOVE AND DISPOSE MANHOLE	DELICITE AND DISCOST LITERAL ASSESSED	PENCIA AND DISDOST CONTRACTOR MARKER	PENCIE AND DISCOSE OBSERVATION WELL	NEWOVE AND DISPUSE PIPE	REMOVE AND DISPUSE PAVEMENT AND RIGID BASE	NEMOVE AND DISPOSE RIGID BASE	REMOVE AND DISPOSE SIGN	REMOVE AND DISPOSE TRAFFIC SIGNAL SYSTEM	REMOVE AND DISPOSE SIDEWALK	REMOVE AND DISPOSE TELEPHONE DUCT BANKS	REMOVE AND DISPOSE UTILITY POLE	REMOVE AND DISPOSE PAVED WATERWAY	FILTER FABBIC RIPRAP FLABED FND LINDERS ASSENT		LOKED GUARDINIL END TREATMENT	MPACT ATTENUATOR	IMPERVIOUS DITCH LINER	LIMIT OF DISTURBANCE	LIMIT OF REGRADING	Carry Will Land	TOOM VALUE SEED	NEW FIRE HYDRANT WITH GATE VALVE	SCALE: NOT TO SCALE	CHECKED BY:	REVISIONS	SHEET: 2 NO. DATE BY NO. DATE	on 2 12/22 RS		
(3.5))(2	(§.1.6))(<u></u>)((F)	(g)	VBV)	9	()(الع	(<u>)</u>	(SAFG))(5	<u>)</u> (E S	(g)	(F)()((8))(3)(g)	(8)	(a	8)(٤	(2	3)((S)		())(<u>ē</u>)(g)(E)(E)	1	z)		(Head)()(2)(2)()(g)(g	<u>s</u>)(NED)	B	(2)	(MMD	(E) (E	<u>)</u> (<u>*</u>		(3)	(SO)	(•)(<u>(</u>	20	ĕ	DATE	ž			
BIDMINOUS BERM							BALED HAY DITCH AND SWALE EROSION CHECK	LOG AND HAY CHECK DAM	DEWATERING BASIN	BALED HAY CATCH BASIN INLET PROTECTION				RUBBLE MASONRY WALL	CONCRETE RETAINING WALL	STONE MASONRY STEPS		CONCRETE HIGHWAY BOUND	POST AND MOUNTINGS FOR RURAL MALBOX	.) POST AND MULTIPLE MOUNTINGS FOR RURAL MAILBOXES	PRECAST TYPE "A" HANDHOLF	DEMONSTRATE AND ADDRESS OF THE PERSON	TOTAL DOLL LIVE IN TANKING INC.	ALUMINUM LIGHTING STANDARDS	BI-DIRECTIONAL CONTROL DEVICE	STREET SIGN MOUNTING DETAIL	TOTAL PLAN THE STATE OF THE STA	COLUMN WITH WALKINGS	PVC PLASTIC PIPE TYPE III BARRICADE	CHAIN LINK FENCE 3'-0" TO 4'-0"	CHAIN LINK FENCE 5'-0" TO 6'-0"	CHAIN INK FENCE 41-0" TO 61-0" MITERIATE BOOT	COLUMN TRUCE 3 -0 10 d =0 INTERMEDIALE POSI	MOVEN WIRE RIGHT-OF-WAY FENCE (STEEL POST)	ROADSIDE GUARDRAIL (GENERAL NOTES, INSTALLATION, POST & OFFSET BLOCK DETAILS)	TYPICAL GLARDRAII INSTALLATION AT STRUCTURES	STEEL BEAM GUARDRAIL ENCASED POST FOR	SHALLOW INSTALLATION	STEEL BEAM GUARDRAL DEEP POST INSTALLATION	STEEL BEAM CUARDRAIL INSTALLED IN CONCRETE OR HIMA SURFACE	STEEL BEAM GUARDRAIL, TL-3	STEEL BEAM GUARDRAIL, TL-2	STEEL BEAM GUARDRAIL DOUBLE FACE ASSEMBLY	STEEL REAL GLIABORALI BEELECTORIZED TRANSLILAS DELINISTED	STEEL BEAM GUARDRAIL REFLECTORIZED TRIANGULAR DELINEATO STEEL BEAM GUARDRAIL ABBROACH END TREATHERT	CTTS OF STREET STREET STREET	STEEL BEAM GOARDROAL TERMINAL END SECTION	SIEEL BEAM GUARDRAIL ANCHORAGE TRAILING END SECTION	STEEL BEAM GUARDRAIL THRIE BEAM TRANSITION PANEL	STEEL BEAM GUARDRAIL CONNECTION TO NEW END POST GLARDRAIL CONNECTION TO EXISTING EMP DOST ADDROGGE	END SECTION GUARDRAL CONNECTION TO EXISTING FUN POST TRAILING	END SECTION	STEEL BEAM GUARDRAIL TRANSITION TO RIGID BARRIER	MASH GUARDRAIL TRANSITION TO EXISTING GUARDRAIL	STEEL BEAM GUARDRAIL LONG SPAN, TL-3	STEEL THRIE BEAM GUARDRAIL SINGLE FACE	STEEL THRIE BEAM GUARDRAIL DOUBLE FACE	STEEL THRIE BEAM GUARDPAIL LONG SPAN	F SHAPE CONCRETE BARRIER DOUBLE FACE	F SHAPE CONCRETE BARRIER SINGLE FACE	F SHAPE CONCRETE BARRIER WITH CONCRETE SEPARATOR		PRECAST MEDIAN BARBRED FOR LICHT STANDARD		BARRIER MOUNTED DELINEATOR	SINGLE—FACED PRECAST MEDIAN BARRIER	PRECAST MEDIAN BARRIER TRANSMON UNIT	BARRIER MOUNTED DELINEATOR	CEMENT CONCRETE SIDEWALK	BITUMINOUS CONCRETE SIDEWALK	WHEELCHAR RAMP	WHEELCHAIR RAMP FOR LIMITED BIGHT-OF-WAY ASSAS	DRATWAY DESTRONG TO 3'-0" TRANSPORT OF THE	DRIVEWAY DESCRIPTION FOR RIVER TEAUGIFUL CORE	UNIVEWAY DEVELOPMENT FOR 6 -0" TRANSITION CURB		RHODE ISLAND		DEPARTMENT OF TRANSPORTATION			
(3.5.1)	(2)	820)(5			3)(9	98	(0.70)	6.8.0	086	(6)((10.2.0)	(F)	10.40)(((\$1.6)	(15.2.0) (NO	18.20)()(93.0	(20.2.0) (20.2.0)	(24.6.)	96.30)(()	61.19	(31.2.0)	(2))(3)(4 .1.0	(1)	()(3	(4.1.4)	G4.20	(34.2.1)	3422) (5))(3)(3)()(g)(9((P)	\$ 20 P	34.5.5	(P)	40.2 ₀	(2)	40.30	(e))()(<u>(</u>	603.0	0.50	(F)	6320	(53.0)	(3))(§)(3				ָרָ בְּיִבְּיִבְּיִבְּיִבְּיִבְּיִבְּיִבְּיִ	אאייי			
INDERDRAIN	CONCRETE CONNECTING COLLAR	CONCRETE HEADWALLS FOR PIPE CULVERTS	STANDARD HEADWALLS FOR MULTIPLE	3'-6" TO 7'-0' PIPE CULVERTS	.) PHECASI CONCRETE FLAMED END SECTION	BRICK/SOLID BLOCK 4'-0" ROUND MANHOLE	.) BRICK/SOLID BLOCK 5'-0" OR 6'-0" ROUND MANHOLE	BRICK/SOLID BLOCK TYPE "D" SQUARE CATCH BASIN	BRICK/SOLID BLOCK TYPE "F" SQUARE CATCH BASIN	SOUD BLOCK FLUSH SQUARE CATCH BASIN	BRICK/SOLID BLOCK TYPE "D" ROUND CATCH BASIN	The state of the s	BRICK, SOUD BLOCK ROUND CAICH BASIN WITH GUITER INLET	BRICK/SOLID BLOCK TYPE "F" ROUND CATCH BASIN	BRICK/SOLID BLOCK TYPE "R" CATCH BASIN	SOLID BLOCK FLUSH ROLIND CATCH BASIN	NIGOT COLOR OF THE PROPERTY OF	(3.4.5) (UM.) BRICK/SOLID BLOCK 5'-0" OR 6'-0" ROUND CATCH BASIN	SOLID BLOCK SHALLOW TYPE "F" SQUARE CATCH BASIN	E) SOLID BLOCK SHALLOW 5'-0" OR 6'-0" SQUARE CATCH BASIN	BRICK/SOLID BLOCK DROP INLET	1370 CHR & BRICK/SOLID BLOCK ROUND MANHOLE OR	CATCH BASIN GREATER THAN 12'-0"	PRECAST 4'-0" ROUND MANHOLE	PRECAST 5'-D" ROUND MANHOLE	PRECAST 6"-O" ROUND MANHOLE	DEFCAST A'-O' OF 6'-O' COURSE MANUAL OF OR STORY	SOUTH BASIN) PRECAST 4"-0", 5"-0", DR 6"-0" ROUND CATCH BASIN	PRECAST CONCRETE DROP INLET	PRECAST CONCRETE DROP INLET LATERAL OUTLET	PRECAST CONCRETE DROP IN ET LONGHIDINA AUTHET	מובנים במינית מינים ווידים במינים מודבים	CAICH BASIN AND MANHOLE STEP	CONCRETE COLLARS	LIGHT-DUTY SQUARE FRAME AND ROUND COVER	HEAVY DUTY SOLIABE FRANE AND BOLIND COARD	6.30 LIGHT NUTY BOUND FALLS AND SOCIAL	CISMI-DUIT RUDIND FRAME AND COVER	HEAVY-DUTY ROUND FRAME AND COVER		SQUARE FRAME AND GRATE	(BICYCLE SAFE)		(Blovoi r. carr.)						TION CURB		_	OUND CATCH BASIN)	SQUARE CATCH BASIN)	CH BASIN)	PRECAST CONCRETE SLOPED FACE CURB (STRAIGHT)	PRECAST CONCRETE SLOPED FACE CURB (CIRCULAR)	PRECAST CONCRETE SLOPED FACE TRANSITION CURB	PRECAST CONCRETE TRANSITION CURB				GRAMIE IRANSIIION COMB	ISTION CURB	GRANITE 2'-0" RADIUS CORNER				GRANITE APRON STONE (FOR ROUND CATCH RASIN)			GRAVITE TRANSITION CURB CARRIED FACE TO STORE FACE)									
(0)) (<u>s</u>	(0.1.0))(=			3)((3.2.1)(DIA.)	979	(25)	(333)	(0.4.0))(=)(2		(**))(999	3.5.1)(SIZE	(38)	(0))(:		(2)	(22)	430 (877)(4.50	4.5.1	(5))(:	a)(6.4.0	(6.1.0))(-)(:) ((6.2.1)	(0.0)	(3)	(632	(8.3))(3)(5)()()(<u>)(</u>	<u> </u>	<u>a</u>)((1)			2.08	72.00	(17)	(227)	7.3.05	(3.00))()(a)((2)	(3.5)	(13 (13)	13.7)(2.7))(°)(=	\(\frac{1}{2}\)	(180)								
NEW					<u>.</u>	- AND	ĭ	Ť	٠	N(Size)SD	(aze milion)	(Length - Sze)	N(SIZE)W	N(SIZE)O	N-4(SIZE)I	N-F(SIZE)	N-#(SIZE)E	-		•	-		• 1	•	•	+	9+	NEW S.H.L. PLAT NO. XX	NEW S.F.L. PLAT NO. XX	ard MAN	, C. S. C.	NEW I.E.B.				≒	- COS	11	•				9	:	*							***************************************				!	1						CHARLES AND ADDRESS OF THE PARTY OF THE PART	Cardon and Cardon Street Contraction						LIMIT OF DISTURBANCE	1						1 Cedar Street	Providence, RI 02903				
	EDUE OF PAVEMENT	BANO	and and	GUARDRAL.	MAILBOX	UILLIT POLE	POLE GUY	LUMINARE	Sign	SUBDRAIN	STORMDRAIN	SANITARY SEWER	WATER MAIN	GAS MAIN	TELEPHONE DIECT	IELEPHONE DOC	ELECTRIC DUCT	PLUG AND CAP PIPE	ABANDONED UTILITY	FLARED END SECTION	HEADWALL	WATER OR CAS CATE	THE PART OF THE PARTY.	CALCH BASIN	MANHOLE	HYDRANT	BASELINE OR CENTERLINE	STATE HIGHWAY LINE	STATE FREEWAY LINE	DEDMANDIT FACTORISE LINE	PERMANENT EASEMENT LINE	TEMPORARY EASEMENT LINE	PROPERTY LINE	CITY OR TOWN LINE		PAVED WATERWAY	CONTOUR LINE	OPEN DITCH	R.I. HIGHWAY BOUND	STONE BOUND	RETAINING WALL	FIELD STONE WALL	BORINGS	FFNCF	WOOD OP BRICK LINE	There are also	DAITO OD STREAM	WETI AND ADEA		BUILDING	FOUNDATION	BUILDING TO BE REMOVED	RAII ROAD TRACKS	CIT AND MATCH	RIP-RAP		CUT SLOPE	FILL SLOPE		ROCK CUT	SPOT GRADE	AREA GRADED TO DRAIN	BALED HAY RI STD 9.1.0	BALED HAY & SILT FENCE	RI STD, 9.3.0	EDGE OF WETLAND	WETLAND PERIMETER	AREA SUBJECT TO STORM FLOW	100-YEAR FLOOD PLAIN	LIMIT OF DISTURBANCE	LIMIT OF CLEARING				-		_	Į Aus				
EXISTING										(skt)su-	(SIZE)D	(Size)s ————————————————————————————————————	(SizE)W		(SIZE)1	(America)	(SIZE)E		11-11-11								8++	EXIST. S.H.L. PLAT NO. XX	EXIST, S.F.L. PLAT NO. XX	EXIST. P.E.B.	9 11 1000	0.11	STAN STAN	TOWN NAME	LOWN NAME								€NO.																		7				<	123 124 125	1		TOO TR. FLOOD BOUNDARY							•	RECEIVED	2772024 no Bridge Company	WASHADISHIPT COLUMN.			

- ANY DAMAGE TO EXISTING PAVEMENT, BRIDGES, DRAINAGE STRUCTURES, PRANKANGE PRES, MELTRACTING MERGAS, ROADSIDE, CONDUIT, SIDEMALK, FENDES FIC., CAUSED BY THE CONTRACTOR SHALL BE REPARED BY THE CONTRACTOR AT NO ADDITIONAL, COST TO THE STATE.
 - - THE OF HET CONTROLLE'S RESPONDED. TO BE SHELD FOR SHELD
 - ALL RJ. STD. 9.9.0 CONSTRUCTION ACCESS ROADS SHALL BE CONSTRUCTED PRIOR TO ANY ROADWAY ACCEPTING CONSTRUCTION TRAFFIC.
- THE FREQUENCY AND APPLICATION RATES FOR THE DUST CONTROL ITEMS WILL BE DETERMINED BY THE CONTRACTOR TO MEET THE REQUIREMENTS OF SECTION 907.
- ALL SIDEMALK AND DRIVEWAYS DESIGNATED FOR REPLACEMENT SHALL BE CUT AND NATCHED AT LOCATIONS SHOWN ON THE PLANS OR AS DIRECTED BY THE PHONETR.
- ERRINAL TRUNCAL ONC CONTRACT BROAD OF THE PLACE BRO
 - THE LUST OF CLASSING AND STREAM CHARGES SHALL BE STREAM CHARGES STREAM CHARGES AND STREAM CHARGES SHALL BE STREAM CHARGES STREAM CHARGES SHALL BE STRE
 - THE CONTRACTOR WILL NOT BE ALLOWED TO STOCKPILE REMOVED PAVEMENT MATERIALS WITHIN THE PROJECT LIMITS.
- 10. CERWING AND SWEEPING OF PAREIGNT WILL INCLUDE REMOVAL OF ALL PAREIGN PAREIGN TO THE PACKERING NO SWEEPINGS AND SWEEPING SHALL BE DONE TO THE STREAGGING OF THE PERSECUENCE WIS SWELL BE DONE TO THE STREAGGING OF WITH THE PRESECUENCE WIS SWELL ONLY BE ALLOWED WITH THE APPRICAL OF THE EDISMECTS.
 - 11. PRIOR TO INSTALLATION, ALL SIGNS, MOUNTINGS AND LOCATIONS SHALL BE AS SHOWN ON THE PLANS AND SHOP DRAWINGS OR AS MODIFIED BY THE ENGINETR.
 - 12. THE COORDINATE SYSTEM, IF SHOWN, IS THE RHODE ISLAND STATE PLANE COORDINATE SYSTEM.
- 13 PAYEMENT OPERATIONS FOR CURBED SECTIONS. IN AREAS WHERE CURBING IS SET TO THISH LINE, MAN DARAGE, HE CONTROLTEN WILL NOT BE RECURED TO THILLE SENSOR AND SIX-TIPE DEVICE FOR AUTOMATIC GABLE COMPRIDED FOR AUTOMATIC GABLE. COMPRISED FOR MAINTAIN ADMINISTRY BETWEEN THE BITUMINOUS PAYER FOR CONTRICLING GABLE.
 - 14. THE COMPACTOR SHALL BE RESPONSIBLE FOR MANYANING ALL ROADWAYS FREE OF DEBRIS RESULTING FOOM THER CONSTRUCTION OFFEATIONS. ALL DEBRIS SHALL BE REAVED TO MANTAIN THE SAFE TRAKEL OF THE PUBLIC AT NO ADDITIONAL COST TO THE STATE.
- 15. NO THE STRONGE, CHRISTIC STRUCKS, ON EGUIPHOLD RETONGES SHALL THAT RECOLUENCES WELL SHALL NESS SHOWN THOS OF METHONG CORP. THO OF METHONG CORP. THOSE OF STRUCKS SHALL ON RECOLUENCE THAT SHALL DAY.
 - 18. THE CONTRACTOR SHALL BE RESCOURDED TO RISURE THAT IT THE BIND OF THAN PARMED OF STREAMS TO A SHALL BE BIND WHITE THE SHALL BE BIND THAN WIN SOURCE DESCRIPTION DESCRIPTION TO SHALL BE NO SERVATE PAYMENT FOR THIS PROVINCE. A PRIVARY COMPETING THE THOS THAN SHALL BE CONSIDERED INCIDENTAL TO PAYMEN OF PRIVARY OF SHALL BE CONSIDERED INCIDENTAL TO PAYMEN AND COLD.
 - 17. AL EMBANGNEN'S SYALL BE PLACED IN HORIZONTAL LAYERS NOT EXCEDING 12. (FORTER COLPACITION) AND SYALL BE COMPACTED SY SEPERIED BEFORE THE NEXT LAYER IS PLACED. ALSO, EMBANGHEN CONSTRUCTION SYALL CONSTRUCTION 15 SECTION 2020.32. OF THE RIJDO.T. STANDARD SPECIFICATIONS, LATES EDITION.
- I. IT THE PROJECT IS ON A HURBICANE EMCLATION AND DIVERSIONARY ROUTE, AS DESIGNATION ON THE CONTRACTORS A ADMINED THAT UPON 12 (TIMELY) HOURS MOTICE THE ROBOWNY SAULI BE OPEN TO EMCLEES AND ELEMENTORY SEGONARY. DAY ODDARY WITH THE PROFINENCE AND TO COMBAY WITH THE REQUIREMENT WILL BE REMAINED TO WHOSE PROPER ACCOUNT PROCEDURES.
- CONTROLLED STATE TROUGHTON THE THAN THE WIND DEFERT TO ALL OF THE PROFESSION, CONDITIONS, NO STRELLUNGS STATED WIN THE PROFESSION, CONDITIONS, NO STRELLUNGS STATED WIND THE EXPERIMENT OF THANDSCHAFF, MANCHEN (RIGHEN), MANCHEN HE WARK CONST OF THE WARK CONST OF THE WARK CONST OF THE WARK CONST ON THE WARK CONST ON THE WARK CONST ON THE WARK CONDITIONS OF THE CONTROLLED TO THE CONTROLLED THE CONTROLLED THE CONSTRUCTION AND NEXT CONTROLLED WINN THE CONSTRUCTION AND NEXT CONTROLLED WINN THE WASCOUNTED BO TREVIOL.

GENERAL NOTES (CONTINUED)

- ALL REPERTOR WOULD WERE SELECTED TO THE OFFICE OFFICE ACCEPTOR WERE SELECTED TO THE OFFICE ACCEPTOR. WE SELECTED THE OFFI THE OFFI WERE SELECTED THE OFFI TH
 - 21. NO UNPOTICETED CONSTRUCTED FEATURE MAY PROJECT MORE THAN 4 INCHES HEADOR THE THISHED GONE OF A TRACERSABLE SLOPE IN A CLEAR ZONE, 4-9, HEADMALL, DRAIMGE INLET, ETC.
- 22. THE REMAINING SECTION OR STUB OF A BREAKAWAY BASE MAY NOT PROJECT MORE THAN 4 INCHES ABOVE THE FINISHED GRADE OF A TRAVERSABLE SLOPE IN A CLEAR ZONE, #.g. SIGN POSTS, LIGHT POLES, FIRE HYDRANTS, ETC.

DRAINAGE AND EROSION CONTROL NOTES

- CONTROLLED RECEIVED TO MARKE WHO THE ESCORE STORM WHEN PROLITION PRESENTS THOSE WHITE SECORE STORM WHEN PROLITION PRESENTS THE PRESENT OF STORM STORM WHITE THE PRESENT RECOVER THAN THE PROGRESSION STORM S
- P. NO LONGERT THE OF ANY COLSTONE ITERATION. A THE WITH WITH WITH STATEMENT AND A LONGERT TO FOR ANY COLSTONE TO SERVICE AND ANY COLSTONE TO SERVICE AND A LONGERT AND A LONGERT TO SERVICE AND A LONGERT A LONGERT AND A LONGER AND A LONGERT AND A LONGERT AND A LONGERT AND A LONGERT AND A LO
 - STORDENE OF WARRAN WANT OF LOTTOR WHAN ENGINATION WITLANDS ON METERS AND WARRAN WAS SITE STORDEN. SHE SHALL HAS SITE STORDEN. WORKING WANT WAS PROPERATE STORDEN. SHE STORDEN STORDEN. STORDEN. STORDEN. SHALLES SHALLES SHALLES SHALLES SHALLES SHALLES. SHALLES SHALLES SHALLES SHALLES. SHALLES SHALLES SHALLES SHALLES. SHALLES SHALLES SHALLES SHALLES. SHALLES SHALLES SHALLES SHALLES SHALLES SHALLES. SHALLES SHALLES.
- THE THE WARN RELUGES PETER BRESS OF PRESENT OF RESERVENCE OF SERVENCE AND THE CONTRIBUTION OF ADDITIONAL SERVENCE AND THE CONTRIBUTION OF THE CONT
 - SURFACE EROSON CONTROL MATTING SMALL BE USED TO STABLIZE FLANTABLE SOIL MADYOR LOWN NALL DITCHES ANALOGAT OF BITTANDES AND WELLAND PERMETTERS, AND ON ALL SLOPES WITHIN WATER QUALITY BASINS. JUTE WESY IN DIOTHES SMALL EXTEND TO AN ELEVATION 2 FEET ABONE THE BOTTOM OF THE INTO:
- SEEDING ON ALL SLOPES 3 TO 1 OR STEEPER SHALL CONSIST OF THE FOLLOWING APPLICATIONS UNLESS CHANGED IN THE CONTRACT.
 - a. SEEDING TYPE I.
- b. ADHESIVE MULCH STABILIZER
- UNNEGETATED SLOPES SHALL NOT BE UNATTENDED OR EXPOSED FOR PERIODS IN EXCESS OF 2 WEEKS OR THROUGH THE INACTIVE WINTER SEASON.
- R. PRINK TO CONSTRUCTION OPERATORS. THE CONTRACTOR IS RESPONSBLE FOR CLEANING ALLOCATION AND TUDNISHED HIS PETS. AND HEN VIRGINAL CLEANING ALLOCATION (HORIZONIAL AND VERTICAL) OF ALL DISTRICE PRES. AND/ON STRUCTURES WHICH ARE TO BE COMMETCED. NAY WARMION FOUND FROM THE PLANS MUST BE BROUGHT TO THE EMBRICIAL TOTAL.
 - ALL DRAIMGE AND UTILITY STRUCTURES WITHIN THE PAVED ROADWAY SHALL BE SAULDSTRU. TO GRADE WITH THE SURROUNDING PAVEWENT PRIOR TO THE WINTER SAULTDOWN.
- 10. DURNG CONSTRUCTION, THE CONTRACTOR SALL & RESPONSED FOR MANANG.
 THE EFFICAT OF THE DRAWING SYSTEM, ONCE CONSTRUCTION IS CONFIDENTIAL THE DOWNWARD REPORTED THE CONTRACTION IS TRESPONSED FOR ELAKING ALL CATCH BASINS AN TUSSIMG ALL PRES OF ANY CONSTRUCTION RELATED DEBRIS AT NO ADDITIONAL COST.
- 11. ONTHE BESN REAL GROUPS, FROW STRICTURES NOT M. TRAKET, LANK UNDER ON PANS ME DEPRESSON OF LOURS HAVE HAVE AND THE CHITES GOVER USE ME LECTHORS SHOW ARE THAL GRANES, THE CORPINCIONS SHALL LACK FRANCES HOLD THE GOVER CONSTRUCTED IN THIS COMPACT OR AS INSECTED BY THE FORMER.
- 12. PROVISIONS FOR CLEARING TO ACCESS OUTFALLS DURING THE CLEANING AND FLUCKING OF THE CLOSED DRAINAGE SYSTEM SHALL STRICTLY ADHERE TO THE PLANS AND SPECIFICATIONS.
- 13. THE CONTRUCTOR SHALL WISHLIAM LEGISORY CONTRICT, DESCRIPTION SHALL WENTER A CENTRAL TO THE CHARLE WAS THE THROUGH TO THE CHARLE WENTER ADDRESS TO THE THROUGH THE CHARLE WENTER A CENTRAL THROUGH THE THE THE THE CHARLE STRUCKER AND THE THROUGH THE THROUGH THE THROUGH THROUGH
- 14 R.I. STD. 8.8.0 PALED STRAW INLET PROTECTION SHALL BE INSTALLED AT ALL CATCH BASINS AND INMETS HINGHOSTS IS DROPGES. DAG SHALL FREMIN IN PLACE UNIT. THE ABUTTHER GROUND SUBFACES ARE STABLIZED.
- 15 WHERE BALD STRAW INLT PROTECTION AND SILF FENCES ARE USED AT CATCH BESINS, THET SHALL BE RELOKED AT THE DEVINEER IN DROER TO PREVENT CLOSENG OF THE INLET.

DRAINAGE AND EROSION CONTROL NOTES (CONTINUED):

РЕОДИИ, ИО 1780, SHETT 1974, PROJUCT ИО. 1764, NO. SHEETS 2022 3 28

- VESTATION AND PETRONE RECOVER HER READED ONCO AND STRUCTOR WHI VESTATION AND STRUCTOR UNESTED AND STRUCTOR OF A CONTRIBUTION OF A CONTRIBU
- 17 THE OF A MY TALKEN S. TO READ IN LOST 11 MISTS OF ALL ENDINGNOON OF THE RESIDENCE AND CONTROLLED WAS THE PROPERTY OF THE RESIDENCE AND CONTROLLED WAS THE STATE OF THE STAT
 - 18 PROFES TO CAMPAGNET OF CONTRICTION OF CONTRICTION AND SEDURITHATION AND CONTRICTION OF CONTRI
 - 19. ALL COMPOST FILTER SOCK, STRAW BALES, SIJT FENCE OF TEMPORARY PROTECTION SHALL TRANN IN PACE UNTIL MA ACCEPTABLE STAND OF GRASS IS: EST-BLISHED. NEEDED, TEMPORARY SEEDING, CAN HELP TO MINAIZE ENGOIN, TEMPORARY SEED WILL CONTORN TO FILDO.T. STANDARD TEMPORARY SEED MIK.
- THE CONTRACTOR MUST REPAIR AND/OR RESEED ANY AREAS THAT DO NOT DEVELOP WITHIN THE PERIOD OF ONE YEAR AND SHALL DO SO AT NO ADDITIONAL EXPENSE TO THE STATE. THE NORMAL ACCEPTABLE SEASONAL SEEDING DATES ARE SPECIFIED IN SUBSECTION LOZ.03 OF THE RID.O.T. STANDARD SPECIFICATIONS, LATEST EDITION.
- 22. AL COSTS ASSICHATED WITH ADHERBOLE TO THE SWAPP SHALL BE CONSIDERED WITH CROSS TORN THE CONSIDERAL MAIN INCLUDED WITH THE CONSIDERAL SEDIL WITH THE WASSICHALD BIOT THOSE. ADMINIST, SEDIL WHO ENGING CONTROLS, SHALL BE REMAILED IN ALCORROWNER, WHITH HE SWAPP BEDONT. THESE ADDITIONAL ITEMS WILL BE PAID AT THE UNIT PROCE TOR THAT BIO TIELA.
- 23. Any observations of illicit connections or discharges to riddy's drainage Network of outfalls shall be reported to the riddy stormater unit majpliates.

UTILITY NOTES:

- EXISTING UTLITES HAVE BEEN SHOWN ON THE PLANS USING THE BEST AWALABL INFORMATION AND JACK PAPOMALE. BULLION, SERVICE CONNECTIONS (ELECTRIC, GAS, TELEPHONE, WITER AND SAWTIAFT) ARE NOT SHOWN. CONTRACTOR IS TO ASSUME SERVICES ARE PRESENT TO ALL BUILDINGS.
- 2. THE CONTROLOR SOW, THE LOCATIONS OF ALL EXERTING DRAWAGE AND ALL DESTRUCTIONS FOR THE LOCATIONS OF ALL EXERTING DRAWAGE AND ALL DESTRUCTIONS OF ALL DESTRUCTIONS OF
 - 4. EXISTING WATER SERVICES SHAL! BE RECONNECTED TO THE NEW WATER MAINS ALL EXISTING UTILITIES TO BE ABANDONED SHALL BE CAPPED.
- UTILITY SERVICE CONNECTIONS SHALL BE MAINTAINED TO ALL EXISTING FACILITIES TO REMAIN.

 - FIRE HYDRAITS SHALL NOT BE RENOVED FROM SERVICE WITHOUT WRITEN AUTHORIZATION FROM THE FIRE DEPARTMENT OR THE WATER AUTHORITY.
- ALL NEW WATER LINES SHALL BE DISINFECTED TO THE SATISFACTION OF THE WATER AUTHORITY IN ACCORDANCE WITH THE SPECIFICATIONS. 8. ALL UTILITY POLE RELATED WORK SHALL BE BY OTHERS.
- THE CONTRICTOR SALL REPORTE T3--(ULA ADMINIST TO THE ROOT TAC.

 (ULCHEAZ-2732) FOR WORSE ARRAND ROOT OWEN THE WESTERLOTHE (DARWAZ,

 LUGHRAN, TS CERPORAT, TOLL CAMPRES CANDENS STANDES A RESULT OF

 FINANCE TO THIS INSERSITECTIVE MARCHIN IN THE TBLD, OR A CRESULT OF

 FORD TO COMMITTE FOR THE NAME OF THE TREAT OF REPULCED AT NO

 ADMINISTRATION OF THE STATE.

RECEIVE

1 Cedar Street
Suite 400
Providence, Rt 02903
401,272.8100

RHODE ISLAND

RHODE ISLAND

DEPARTMENT OF TRANSPORTATION

EAST BAY BIKE PATH BRIDGE REPLACEMENT (BARRINGTON RIVER AND WAREN BRIDGES) | REVISIONS | REVISIONS | NO. DATE | Br | NO. DATE | Br | NO. DATE | Br | 12/22 JRP | 2 3/14 MLP | 12/22 JRP | 3 4/14 MLP | 12/22 JRP | 3 4/14 MLP | 12/22 JRP | 3 4/14 MLP | 12/22 JRP |

STANDARD NOTES - 1

0188A_V1_003_STDNOTESO

LANDSCAPE NOTES

- ALL PLANT MATERIAL MUST BE TAGED AT THE MURSERY (A RECOGNIZED GROWES OF THE RELLOCI. STANDARD SPECIFICATIONS, LATEST EDITION, ALL PLANT MATERIAL WIST BE MURSERY GROWN NO PLANT MATERIAL WILLS BE MURSERY GROWN NEW PLANT MATERIAL, WILL BE ACCEPTED.
 - ALL PLANT SUBSTITUTIONS AND/OR CHANGES IN PLANT LOCATION MUST BE EAPHONIE IN ACCORDANCE WITH THE R.I.D.O.T. STANDARD SPECIFICATIONS, LATEST EAPHONIE IN ACCORDANCE WITH THE R.I.D.O.T. STANDARD SPECIFICATIONS, LATEST APPROVED.
 - all plant material is to be field located by a representative from the rilo.1. Landscape architecture unit,
 - COORDINATE WITH THE RILD.O.T. CONSTRUCTION MANAGER PRIOR TO ALL TRIMMANS. OLLERING NECESSARY TO COMPLETE THE WORK AS SHOWN ON THE PULMAS.
- ANY TOPSOR, USED AS PLANTABLE SOIL SHALL HAVE A SANDY LOAM TEXTURE STANDARDLY PROFILES, RODS, LUMPS OF SOIL, TREE LIMES, TARS FOR CONSTRUCTION DEBRIS, AND SHALL CONFORM TO SECTION M.18 OF THE R.LID.O.T. STANDARD SPECIFICATIONS, LATEST EDITION.

 - all trees and shrubs shall be mulched with pine bark mulch in Aggordance with the Rild.O.T. Standard specifications, latest edition.

ALL TREES AND/OR SHRUBS THAT ARE PLANTED AS A BED SHALL BE MULCHED AS A BED.

- PROVIDE A MINIMUM 6"—8" BRANCHING STANDARD ON ALL TREES INSTALLED ADJACENT TO SIDEWALKS AND/OR PEDESTRIAN ACCESS AREAS.
- THE CONTRACTOR SHALL PROADE CERTRICATION THAT THERE ARE NO CONTAMBANIS HAT EXCEED THE R.L.D.E.W. PERMISSIBLE LEVELS IN THE SOILS USED AS LOAM OR PLANTABLE SOIL.

STRUCTURAL NOTES FOR HIGHWAY SIGNS, LUMINAIRES AND TRAFFIC SIGNALS;

ALL SUPPORT DESGNS AND ASSOCIATED SHOP DRAWING REVIEWS SHALL BE IN CONCENSACE WHITE LATES THE LANGE OF THE ASSETT OF THE LANGE OF THE L

CONSTRUCTION DRAWINGS AND DETAILS

- THE FOLLOWING NOTES SHALL BE INCLUDED ON ALL PLANS AND/OR SHOP DRAWINGS IN REFERENCE TO ANCHOR BOLTS.
 - "PRETENSIONING OF ALL ANCHOR NUTS IS REQUIRED, AND SHALL BE ACCOMPLISHED BY TIGHTENING TO 1/6TH TURN BEYOND THE SMUG—TIGHT POSITION." "THE MAXIMUM CLEARANCE BETWEEN THE BOTTOM OF THE LEVELING NUTS AND THE TOP OF THE CONDIRETE IS OFFICIAL AND SHALL NOT EXCEED THE AMOUNT SPECIFIED ON THIS DRAWING."
- THE USE OF CROUT UNDER BASE PLATES SHALL CEMERALLY NOT BE FEMILIAD. IS SECRET, CONDITIONS WHRANN ITS USE "THE GROUT SHALL WAT BE CONSIDERED UND CHRANNE ITS USE. THE CROUNT SHALL BE DEPOYED.

 SUPPORTED BY THE ANCHOR BOLTS. ADCOUNTE DRAINAGE SHALL BE PROFINED.
 - THE DAMPENING EFFOTS OF VIBRATION MITIGATION DEVICES SHALL NOT BE CONFIDENCIAL SUPPORTIS FOR SIGNS AND THE WASHINGT FOR THE CHARGE AND THE THESE DRIVINGS THE WASHINGT FOR THE CHARGE STATES THE STATES ASPROVED OF THE DEVIKTIBLE FROM TO AMBRICATION OF SUPPORTS.

THE FINAL POSITION OF SIGNAL HEADS, PEDESTRAIN PUSHBUTTONS, DETECTORS, AND STOP LINE AND PROSSINGER PARKET MERRINGS THE EAGURETED BY THE EAGURETE IN THE TELD ACCORDING TO ACTUAL MITRESCEND CHARACTERSTICS.

A 2" MINIKUM BUFFER SHALL BE PROVIDED BETWEEN THE CURB AND ALL LATERAL OBSTRUCTIONS (INCLUDING ALL SIGNAL POLES AND TRAFFIC/PEDESTRIAN SIGNAL HEADS) TO PROVIDE ADEQUATE CLEARANCE FOR TURNING VEHICLES. ALL FOUNDATIONS MUST HAVE CONES OR BARRELS BOLTED TO FOUNDATION BASES UNTIL ACTUAL POLE IS INSTALLED.

- ALL SALVAGED TRAFFIC SIGNAL EQUIPMENT SHALL BE DELIVERED TO THE RLID.D.T. MANTENANCE HEADOLWRIPES, SO UNGOON AFROLE, SHAWRICK, RHODE ISLAND, DZBBET HE GOOR ELEVERT IS CONSIDERED INCIDERIAL, TO THE WORK.
- MAINTENANCE AND PROTECTION OF TRAFFIC NOTES
- ALL SIGN MOUNTINGS FOR TEMPORARY AND CONSTRUCTION SIGNS SHALL BE IN ACCORDANCE WITH THE R.I.D.G.T. STANDARD SPECIFICATIONS, LATEST EDITION.
- THE CONTRACTOR SHALL COVER ALL EXISTING AND/OR TEMPORARY SIGNS THAT ARE NOT RELEVANT TO THE TRAFFIC CONTROL REQUIRED DURING ANY PARTICULAR STAGE OF THE CONTRACT.
- ARROW PAMELS SHALL BE SET IN THE FLASHING FOUR CORNERS CAUTION WODE UNLESS UTILIZED FOR A MERCING TAPER, ARROW PAMELS SET IN THE FLASHING ARROW MODE SHALL NOT BE UTILIZED FOR LANE SHITS.
- THE INTEGRAL VANCEL FAIRS ARROUNDED FOUNDED FOR COLUMN TO ME COLUMN THE MESSAGE AND COLUMN THE MAN THE MAN THE MESSAGE AND COLUMN THE MAN THE MESSAGE AND COLUMN THE MAN THE MESSAGE OF MAN THE MAN THE MESSAGE OF MANNER, AT THE MESSAGE OF MAN THE MESSAGE OF MANNER, AT THE MESSAGE OF MESSAGE OF MANNER, AT THE MESSAGE OF MESSAGE OF MANNER, AT THE MESSAGE OF MESSAGE OF MANNER, AT THE MESSAGE OF MANNER, AT THE MESSAGE OF MESSAGE OF

TRAFFIC SIGNAL NOTES

| TEL No. | State | TEDEMA, AD | TECH | SHETT | TOTAL | SHETT | SHETT

- BACK PLATES SHALL BE INSTALLED ON ALL TRAFFIC SIGNAL HEADS.
- AL MANTENNEE AND PROTECTION OF TRAFFIC CONTROL SETUPS, SIGNS, CHANNELLING DEPARCE, NET, SHALL BE IN ACCORDING. WITH THE MANUAL ON UNIFORM TRAFFIC CONTROL DEPARCES, LATEST EDITION. THE CONTROL OF SAME AND RETALL ON THE UPPER LEFT HAND CORNERS
 GORDING AND ALE DEPENDENT OF THE THE THE CONTROL ON A CONTROL OF THE CONTROL OF
- ANY TRANSIT PARTIES OF SHORE (NOT 2) WHALE OF USED IN ADJUNCE OF MAY THAN TO WHICH A ELECTRON WITHOUT OF THE WASHINGTON OF THE WITHOUT OF THE WASHINGTON WITHOUT OF THE WASHINGTON OF THE STATION.

TRAFFIC CONTROLLER CABINETS, UNLESS OTHERWISE NOTED, SHALL BE NEVA 152 TYPE 1 CABINET SIZE 6 ("P" TYPE) WITH NOMINAL DIMENSIONS OF 52"HA44"HY-24"D. ALL DELAY AND EXTENSION TIMES, AS CALLED FOR ON THE PLANS, FOR PROPOSED LOOP DETECTORS SECOND STATEMENTS. IN THE TRAFFIC SIGNAL CONTROLLER AND MOT THE DETECTOR RELAY. INSULATED GROUND WIRE SHALL BE PLACED IN ALL PAC CONDUTS AND SHALL BE BONDED TO GROUND RODS IN ACCORDANCE WITH SECTION 1.03 OF THE RHODE BRIDGE CONSTRUCTION.

- POLICE OFFICERS AND FLAGPERSONS SHALL BE UTILIZED AS OUTLINED IN SECTIONS 913 & 914 OF THE RI STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION. POLYTHICHE FIGURE SHALL BE UTILZED AS A CHANNELLEING DEVICE WHEN A TREPTEC DOWNER BETWEEN THROUGH SHALL BE IN WORKEN HE FREE TO THE THROUGH SHALL BE UTILIZED WHEN A TRAFFEC SUBSECUENTLY BROKEN DOWN AT THE UTILIZED WHEN AND IS SUBSECUENTLY BROKEN DOWN AT THE USE OF THE WORKEN.
 - TEMPORARY CONSTRUCTION SIGNS AND OTHER WORKZONE TRAFFIC CONTROL BODYCES THAN ATE DAMAGED OR REQUIRE RELOCATION SHALL BE REPLACED AND / OR RELOCATED UNDER THE PAY ITEM FOR "MANITEMANCE AND MONEMENT TRAFFIC PROTECTION."
- THE PRIVATE VEHICLES OF CONSTRUCTION WORKERS SHALL NOT BE PARKED. ON THE TRANSEL LANESS OF SOLULISES. THEY MAY BE PARKED WITHIN THE STATE BIGHT-OF-WAY ONLY IN AREAS BEYOND THE OUTSIDE EDGE OF THE TRANSEL LANES AND/OR IN AREAS APPROVED BY THE ENCHARER. TEUPORARY CONSTRUCTION SIGNS AND OTHER TEUPORARY TRAFFIC CONTROL BOYCES SHALL BE INSTITUTED PRIOR TO THE START OF WORK IN ANY AREA OPEN TO TRAFFIC, AND SHALL BE REMOVED AS SOON AS PRACTICAL, WHEN THEY ARE NO LONGER APPROPRIATE.

ALL PEDESTRAN PLANEUTONS SHALL BE COMPLIANT WITH THE AMERICANS WITH RESERVED AND ACCOUNTY OLD LIGHTS CONTROLLS OF RELIGIORS. TO CASE PRESSURE ALTONED BRITON. SIGN CONTROLLS OF ACCOUNTY OF ALTONE PROPERTY OF A CONTROLL ACCOUNTY OF A CONTROLL ACCOUNTY. ON THE ACCOUNTY OF A CONTROLL ACCOUNTY OF A CONTROLL ACCOUNTY OF A CONTROLL ACCOUNTY. ON THE ACCOUNTY OF A CONTROLL ACCOUNTY OF A CONTROLL ACCOUNTY OF A CONTROLL ACCOUNTY.

WHEN PLACING TRAFFIC SIGNAL HANDHOLES OR CONDUIT IN EXISTING FORTLAND CLARGIT CONCRETE SIGNAL SEPLACED IN ACCORDANCE WITH R.I. STD. 43.1.0. NO PATCHES WILL BE ALLONED.

ALL LOOP DETECTORS SHALL BE UJT INTO THE FINAL PARENET SURFACE COURSE.
 TRAPTC SIGNAL CONTROLLERS AND CABINITS SHALL BE PROGRAMED, AND WRED SO THAT ANY FIRE PRE-EMPTION SHALL OVERRIDE MANUAL (PUSH BUTTON) OPERATION.

ALL LOOP DETECTORS SHALL BE CENTERED WITHIN EACH LANE AS DELINEATED, UNLESS OTHERWISE DIMENSIONED ON PLANS.

16. PRE COMPACIDN SAML, WORK COMPAUDUSIY TO RESTORE TRAFFIC SIGNAL CREATION TO TAIN MEDICAL PROPROSE WAS INTERFACE SOME CREATION TO TAIN MEDICAL SIGNATION TO MAKET TRAFFIC SIGNAL AT HIS WITESTED WAS ALL MISS. SHOULD MEDICAL SIGNATION TO MAKE THE SAME THE SAME TO RESTORATE THE CONTRACTOR LEAVE THE STORE SETTING RESTORATE THE THE STORE SETTING STORE SAME TRAFFIC OF SAME TO SET SAME THE SAME THE SAME SAME THE SAME THE SAME THE SAME SAME THE SA

RECEIVED A STATE OF THE STATE O

1 Ceclar Street Suite 400 Providence, RI 02903 401.272.8100

RHODE ISLAND

COT DEPARTMENT OF TRANSPORTATION

DESIGNED BY: CHECKED BY: DATE: SHEET: 4

EAST BAY BIKE PATH BRIDGE REPLACEMENT (BARRINGTON RIVER AND WARREN BRIDGES) STANDARD NOTES - 2 0188A_V1_004_STDNOTESO

JOB SPECIFIC UTILITY NOTES R-1 Rt Rt Rt Rt Rt Rt Rt R	1. EVSTMO. UTLITES HAVE REEN SHOWN ON THE PLAKS LISHO, THE REST AMALUAE. N. ECHNIFACTINE WHI HE APPENDANCE LUCHARION OF THE EXPENTES WILL BE CHECKED BY THE CONTINUATION WHI HE APPENDANCE UTLITY COMPANIES. ALL LUCTINUA MANUALES, CACHE BASINS, ROCHEWER TORSE, AND STRONG ALL UTLINES WITHIN THE RESIDENCE WHOSE MALL DE FALLOWERS WITH THE RESIDENCE WHOSE MALL DE FALLOWERS WITH THE RESIDENCE WHOSE MALL BE FALLOWERS.	ri s		⊸i	.÷ 6	REPURD REQUEST OF VOILT THE COMPREDIME AREA HAS BEDY STREIGIZED. THE INLET SERVED CONTROL TO VOILT THE COMPREDIME AREA HAS BEDY STREIGHT ON AS DIRECTED BY THE REMOVED AT THE THO OF THE PROJECT ON AS DIRECTED BY THE REMOVED TO DRAWING SYSTEMS. **CONCRETE WHICH PROVIDE A COMPRETE WASHOUT AFEA.				
JOB SPECIFIC GENERAL NOTES	1. AL TIENS FOR PERBENCED FOR MODIFICATION OF THE PLANS OR IN THE CONTRACT DOCUMENT'S THE ET EXISTEN OF PERMINE TUNESS ORIGINATE DRIFTED BY THE LIGHTON OF PERMINE TO FEATURE THE PLANSES OF THE TRACT DOCUMENT AND METRICAL SHALL BE PLANSED AN ADSCURE DISTANCE OF THE TRACTION OF THE TRACT	3. THE CONTRACTOR SHALL TAKE SEESAL CARE NOT TO DAMAGE ANY DISERTOR VEGETATION WHICH OPERANGS OR IS ALALGEN'T TO THE CONSTRUCTION ZONE. ANY DAMAGE THAT IS CHARGE OF THE CONSTRUCTION ZONE. ANY DAMAGE THAT IS CHARGE OF THE CONTRACTOR'S NEGLECORE LIFE OF THESE ANY DAMAGE THAT IS CHARGE OF THE OFFICE OF THE CONTRACTOR'S AND THE OFFICE OF THE OFFICE OFFICE OF THE OFFICE OF THE OFFICE OFFIC	່ and u	, r	LILY CANNON ACCESS AND ACCESS AND THE REGISTED FOR THE REGISTED FOR THE PROJECT FOR ACCESS AND ACCESS AND ACCESS ACC	EXCRETE. CONCRETED AND INCOME. OF THE EXCRETATION OF THE SUBSACTION OF THE PROPERTY OF SUB-ALL GROUNDLE CONCRETE AND AS DIRECTION OF THE PLANS AND AS DIRECTIO OF THE PLANS AND AS DIRECTIO OF THE DIAGRAM, SUPPLIES FRICTS FROM SUBSACTION OF AND ASSOCIATED OF THE DIAGRAM, SUPPLIES FRICTS TRANSPORTED AND ASSOCIATED OF THE DIAGRAM, SUPPLIES FRICTS ASSOCIATED AND ASSOCIATED OF THE DIAGRAM, SUPPLIES FOR SOIL SEPARATION AND BE ONE OF THOSE INCOURTED ON REDOY'S PREPROPER.	MANDERS DESIGNATED SCALES SHALL BE PROPERLY REMORED, HAULED AND DISPOSED AS REQUIRED. 11. FOR DEMORSTRANCE HAIL WE PER DISPOSED FOR EXCAMPINA AND INSTILLATION OF ABITHERITS AND RETAYMENT BALLS, SEE PLANSET FOLIABLE.			
JOB SPECUPIO, LEGENDIFICATIVATI): (450) LORIGORIE TRES POR 47 PLANTERS (450) CORRESPONDE TRES POR 47 PLANTERS			9 T CASS I SHAM. 12" GRANEL BIORNAN BLIBBABE COLREE RELIVOE AND RESET SIGN TEMPORARY LMIT OF RESEAUCHG GUANNO TEMPORARY PLATFORM CONSTRUCTION)	JOB SPECIFIC PLAN SYMBOLS	EXISTING COMPOST FILTER SOCK COMPOST FILTER SOCK CONTRACTOR	TMBER AND COMPOSITE PEDESTRIAN BRIDGE APPROACH RAIL	GUALIFED PERVOUS AREA STONE REPEAP AT TEMPORARY BULKHEJÖ	DUMPED RIPRAP TO SUPPLEMENT ENSTING RIPRAP	PACKED RIPAR PROTECTION FROM PROPOSED ABJUNENT TO EXISTING ABJUNENT TO EXISTING	

1 Cedar Street
Suite 400
Providence, RI 02903
401,272.8100

RHODE ISLAND

RHODE ISLAND

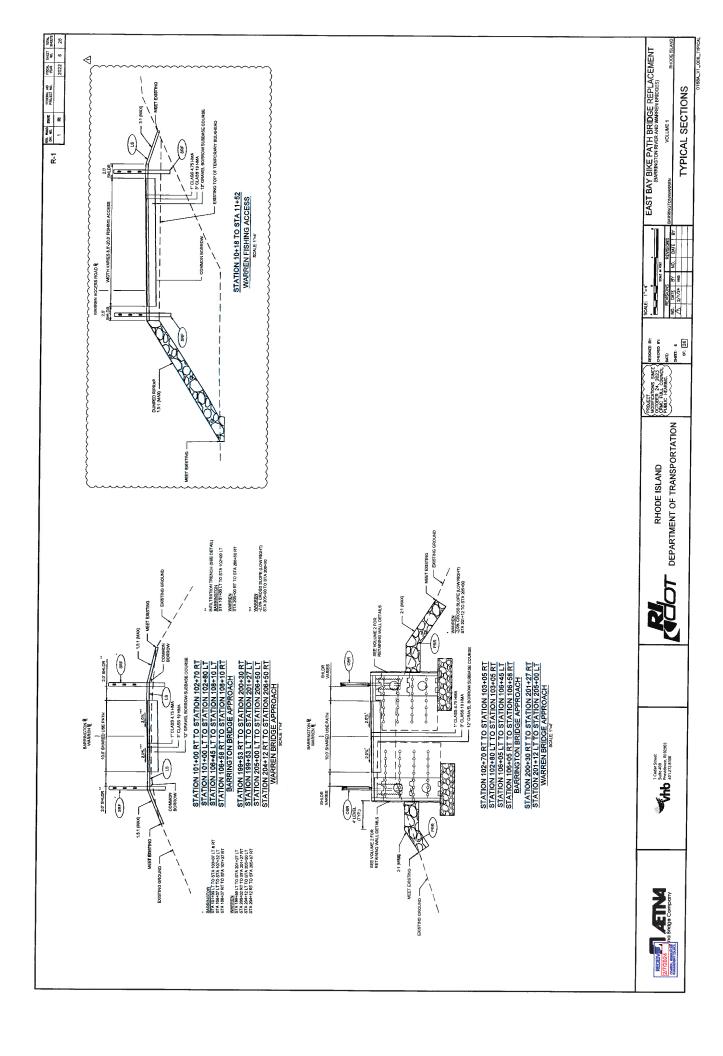
PROPERTMENT OF TRANSPORTATION

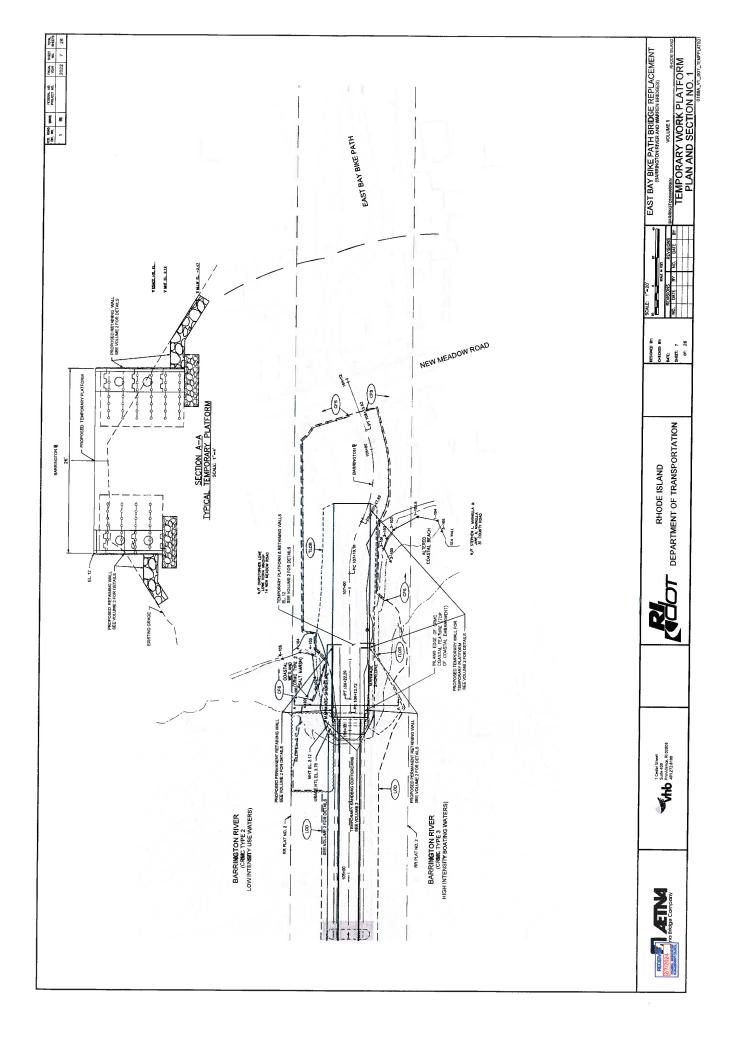
EAST BAY BIKE PATH BRIDGE REPLACEMENT (BAREN HIDGE)

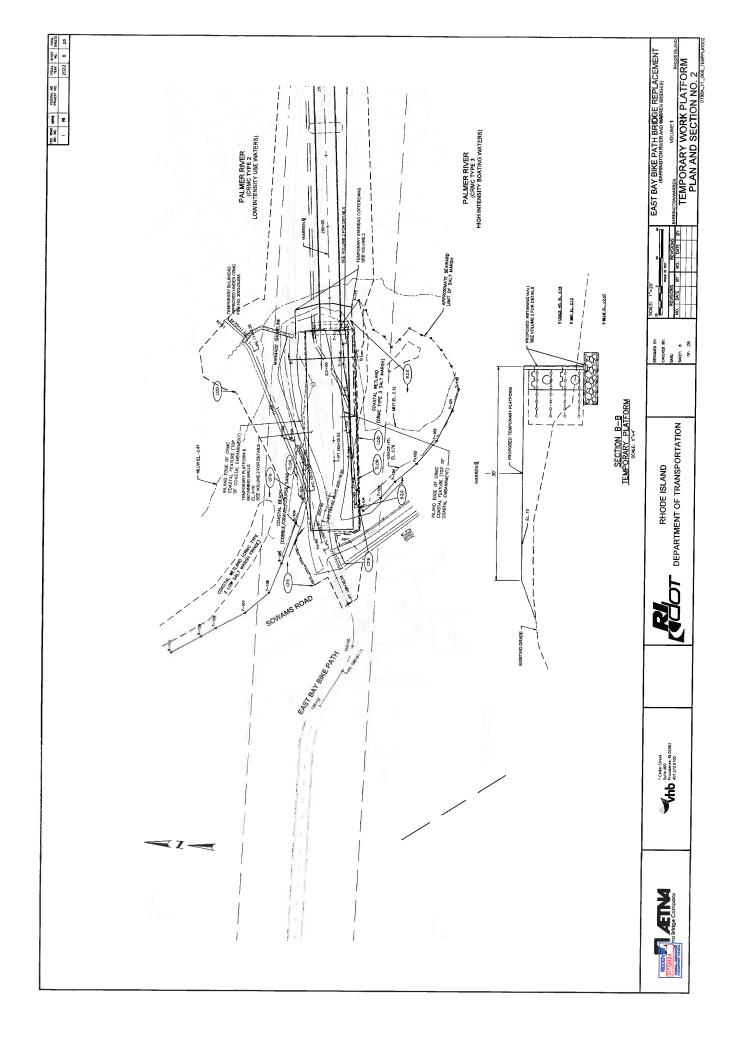
PLAN SYMBOLS, LEGEND & NOTES

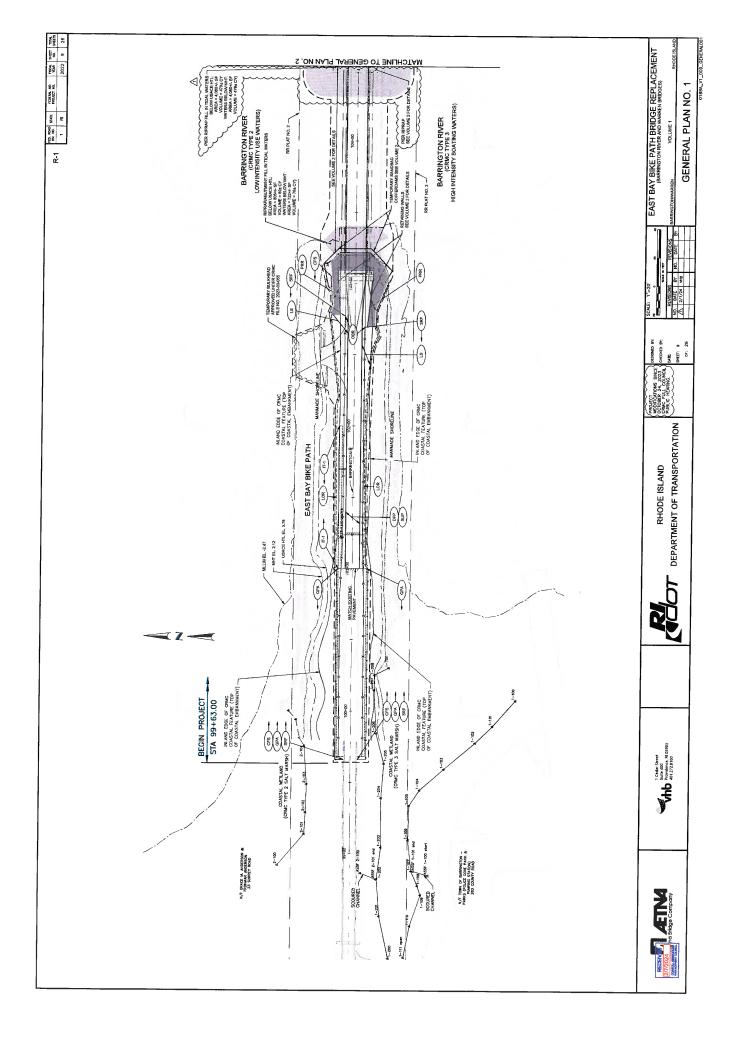
PLAN SYMBOLS, LEGEND & NOTES

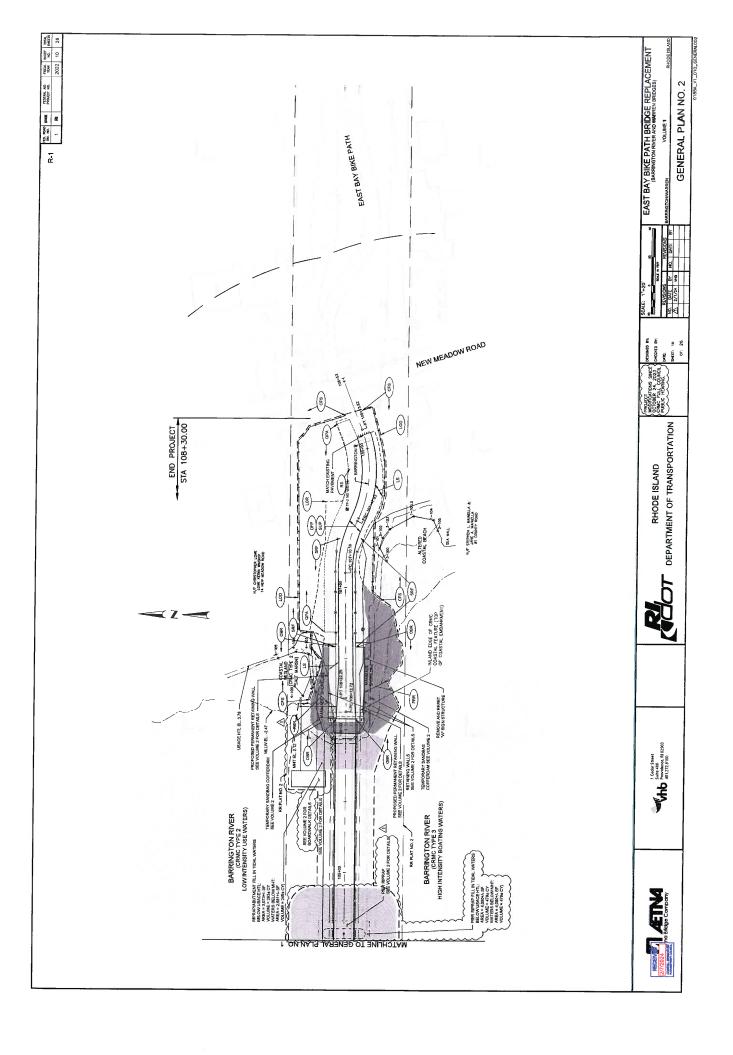
OTHER STATES OTHER STAT

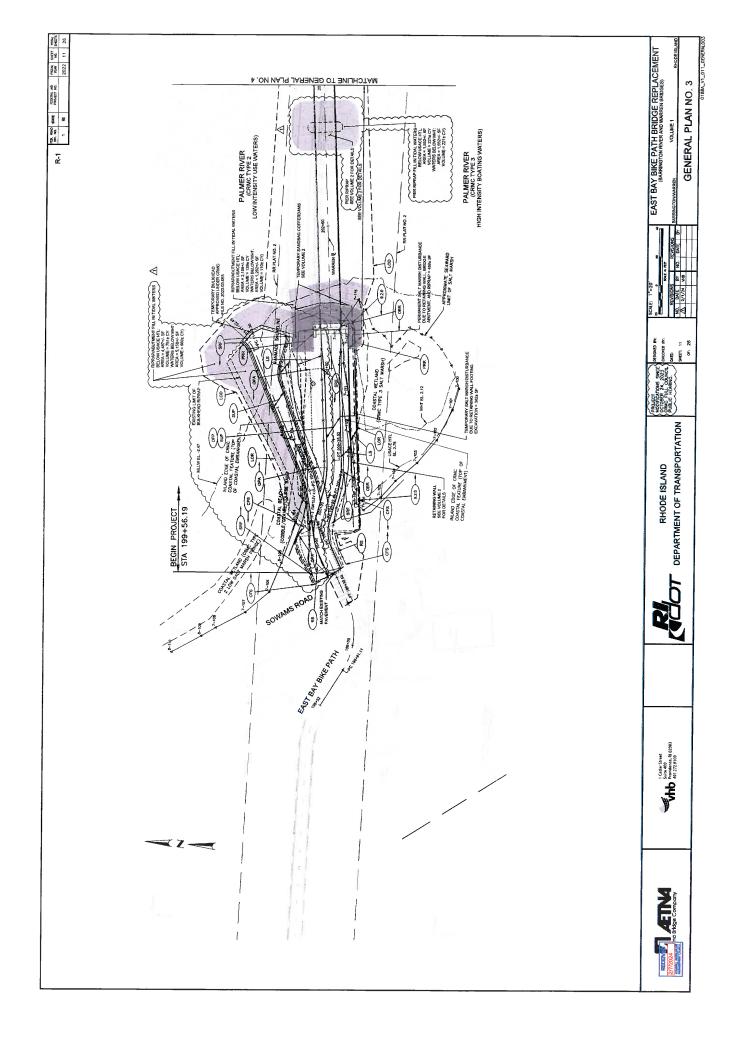


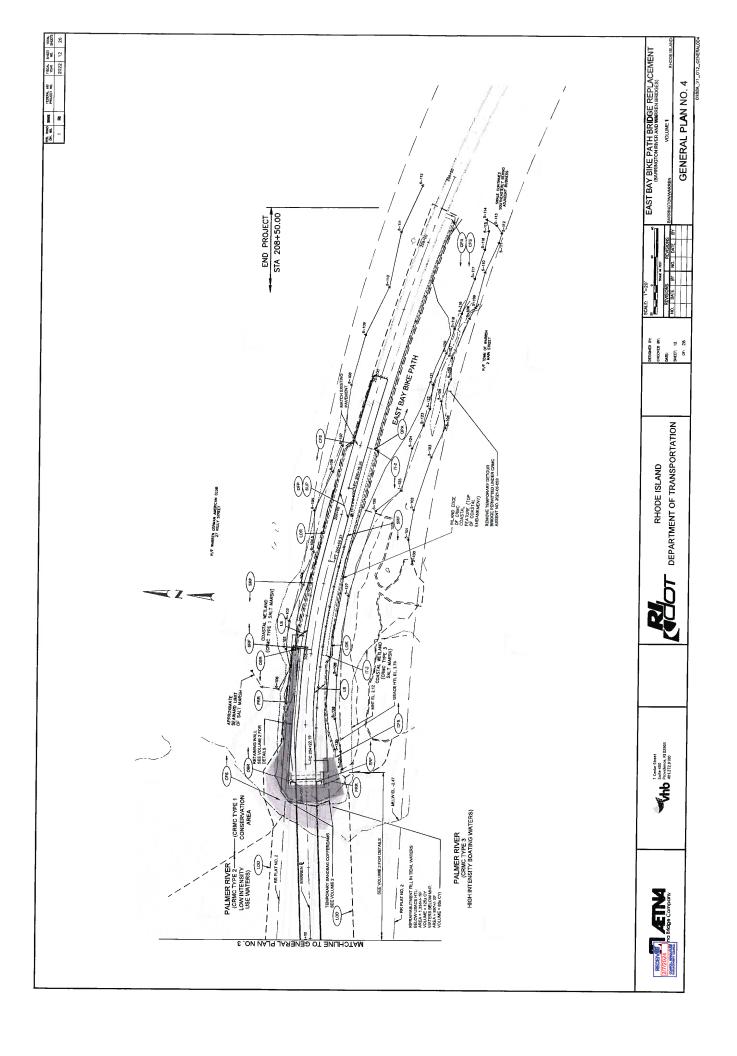


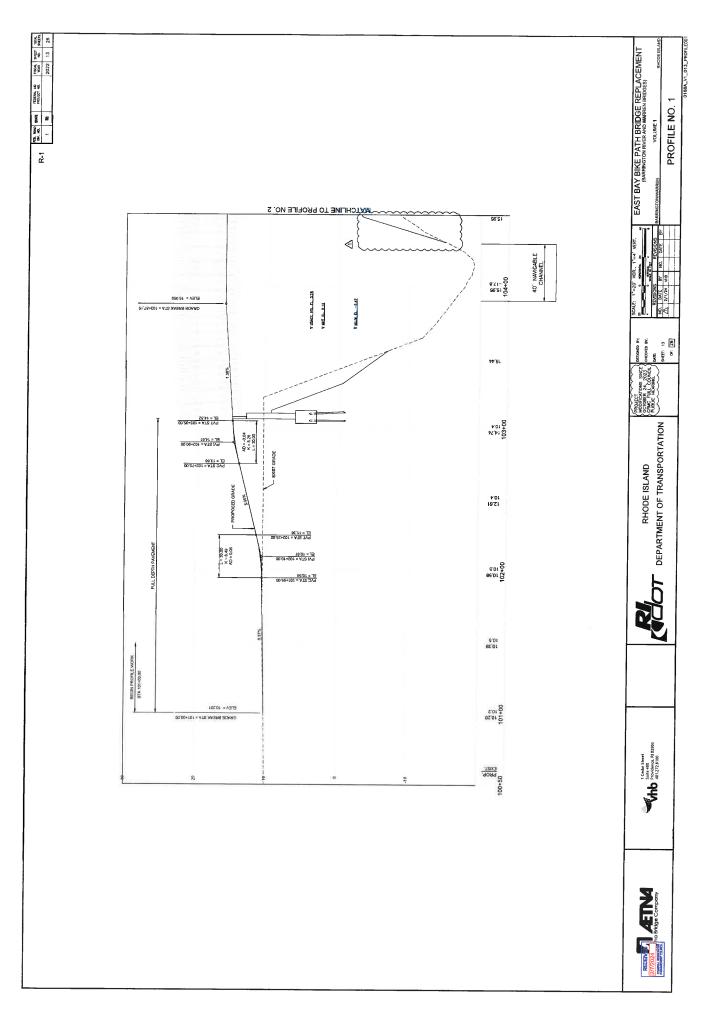


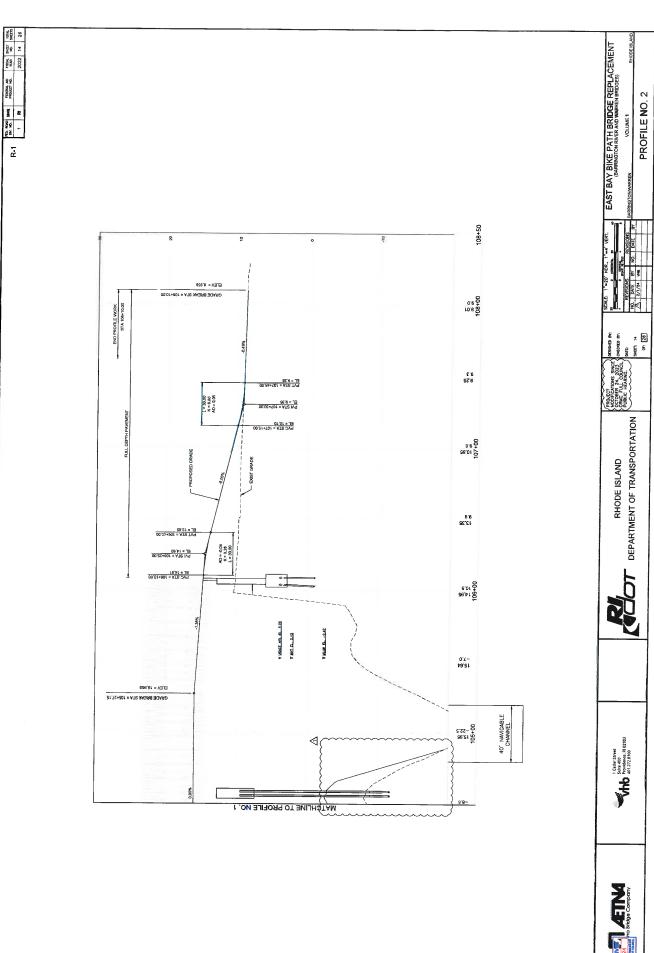




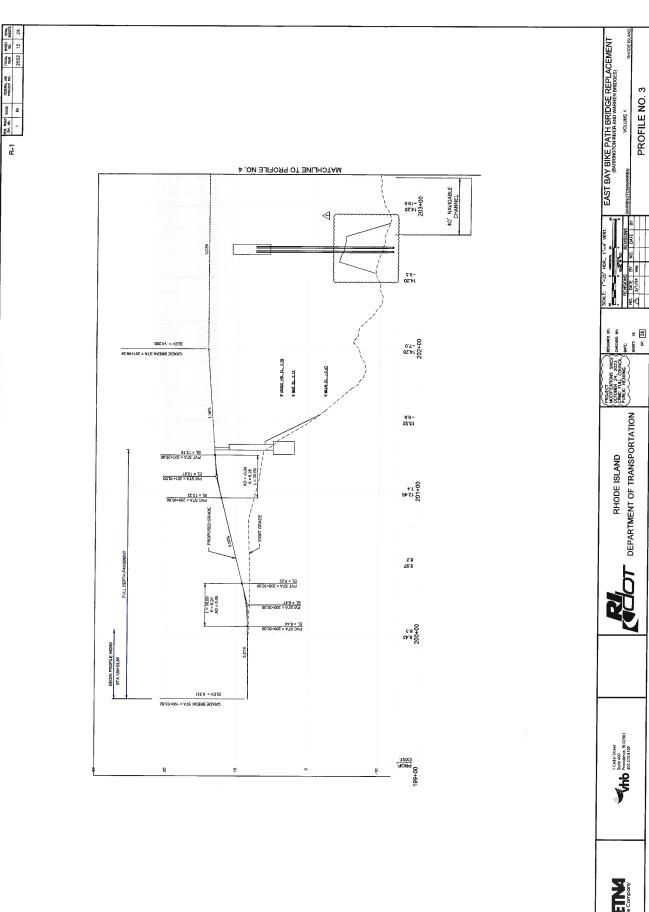




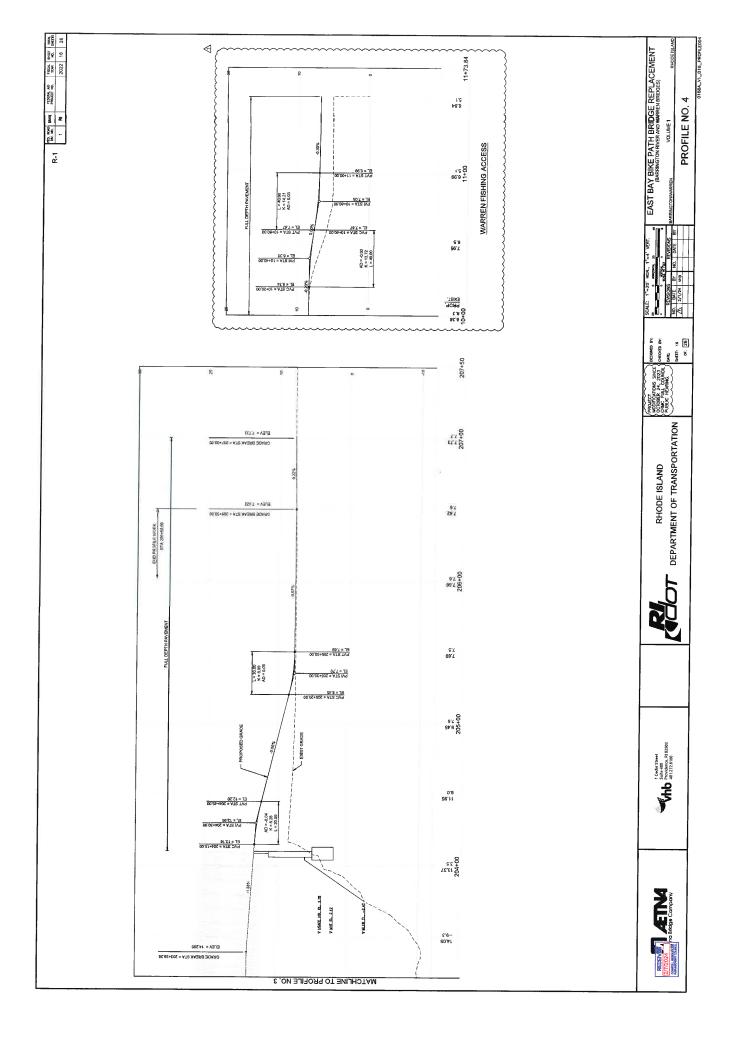


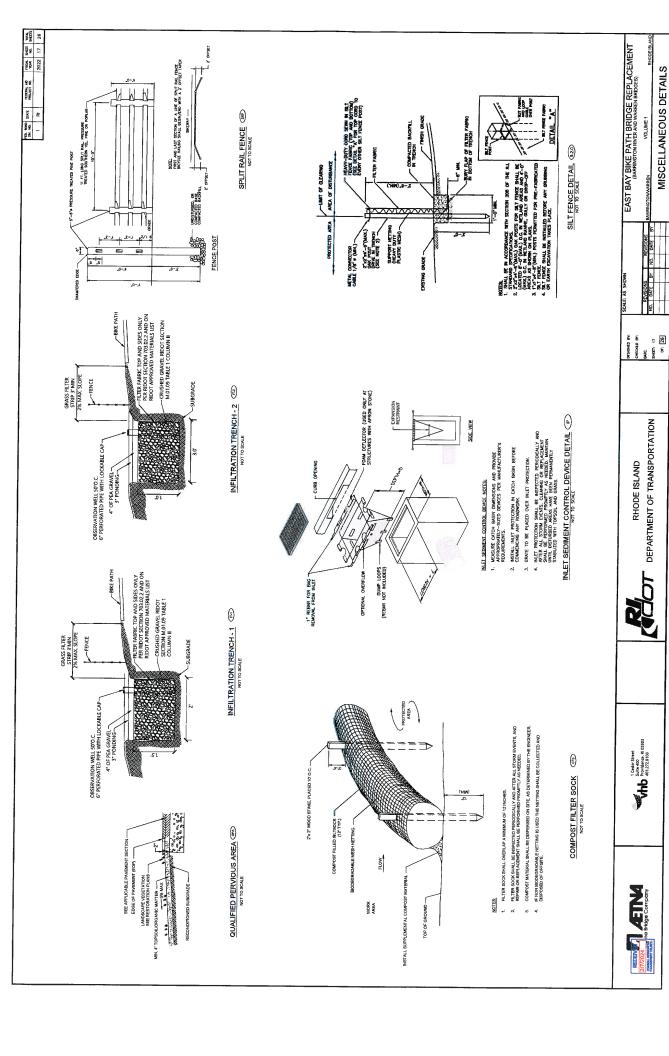


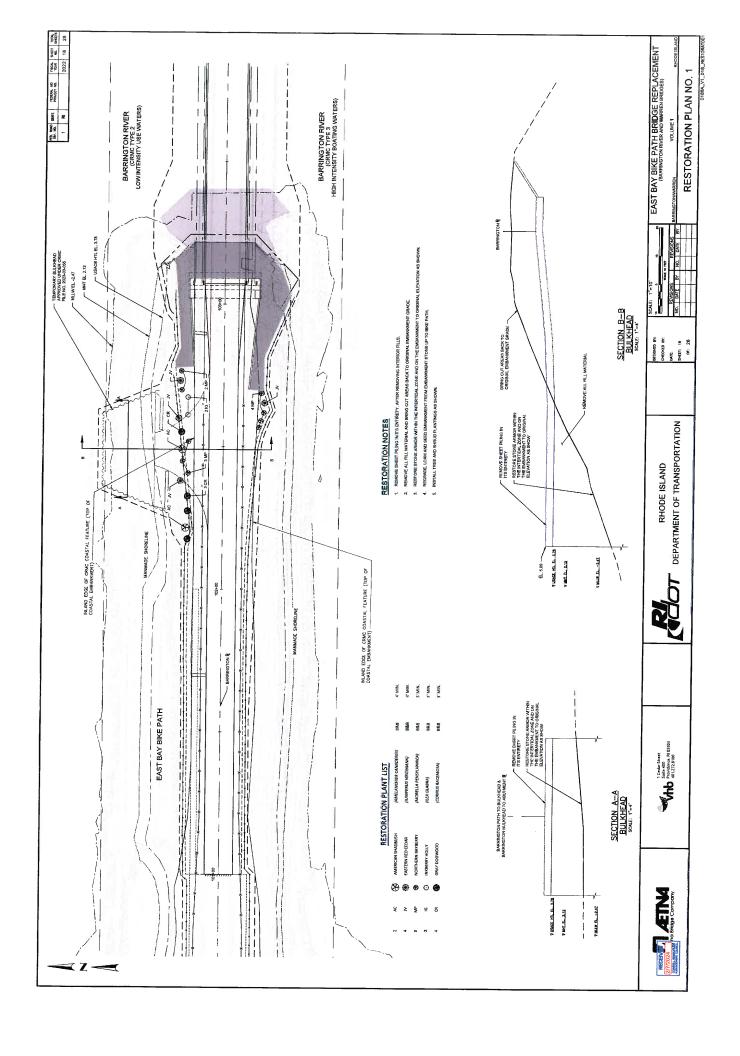


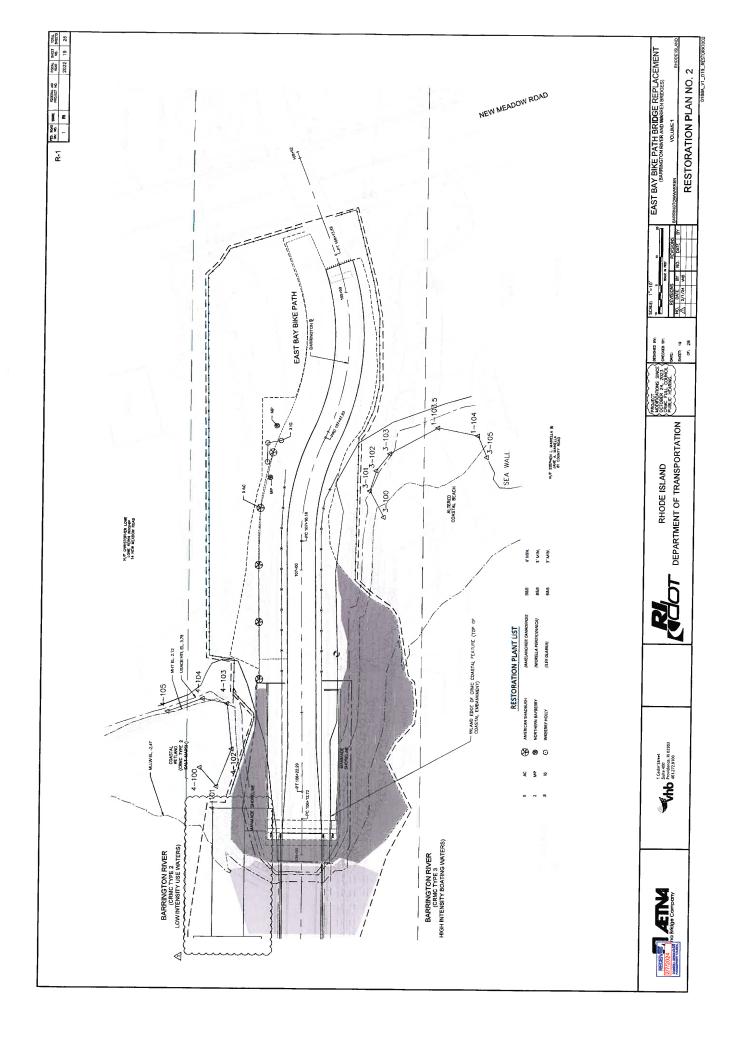


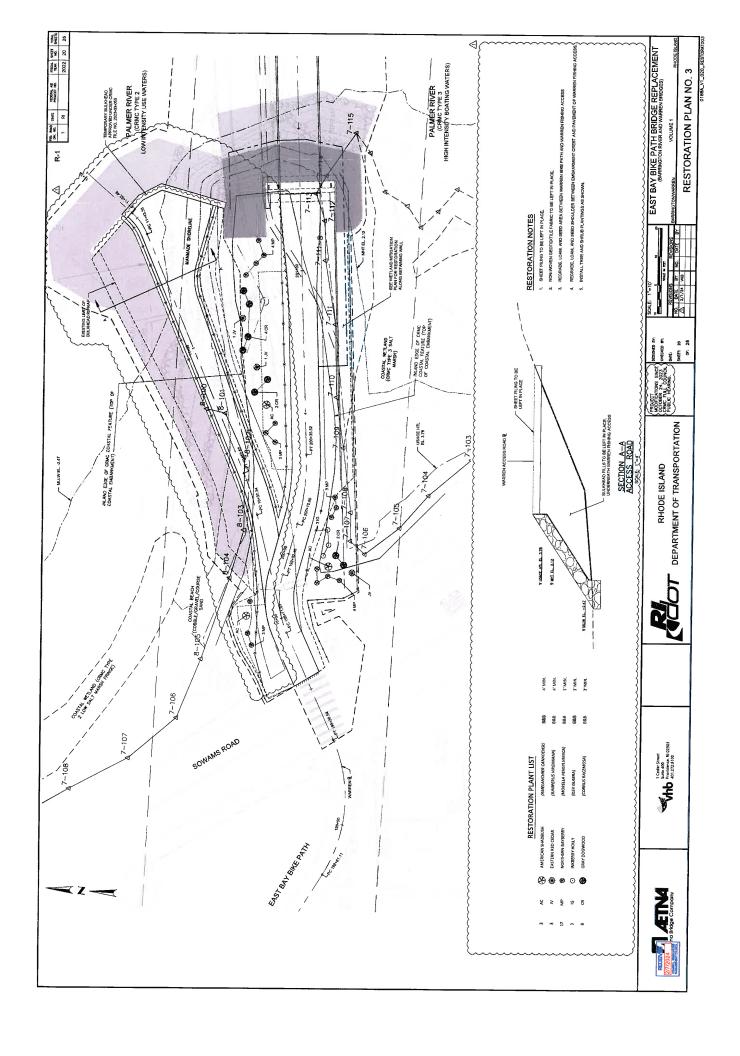
MESSIVE TO BHOUSE COMPONY

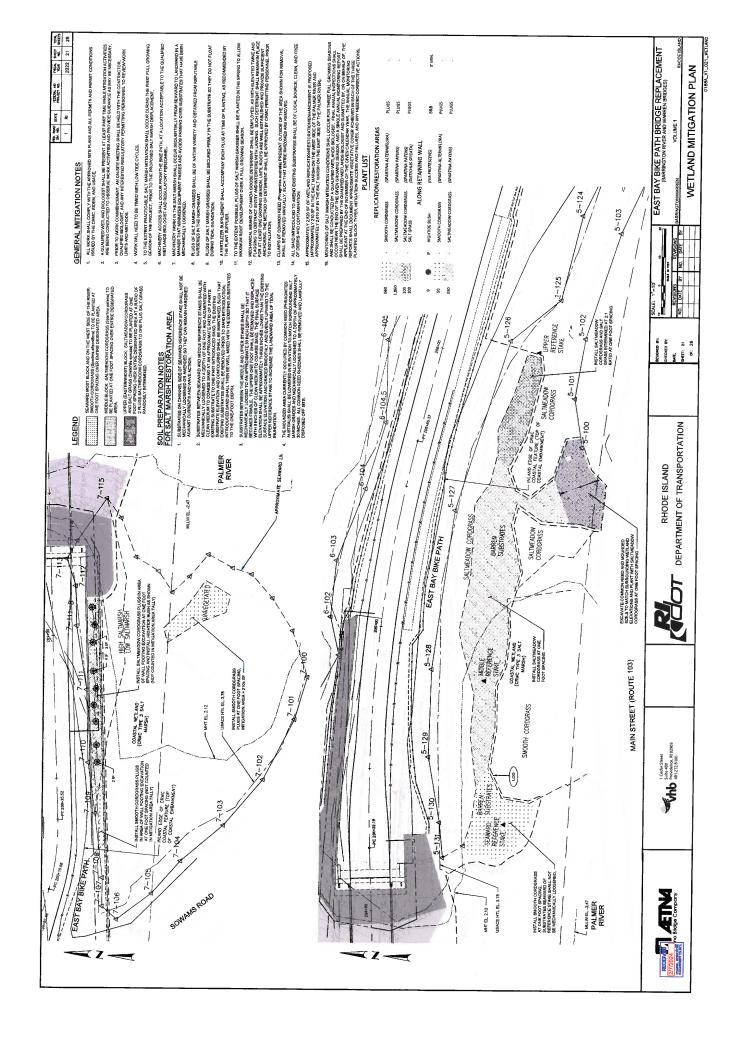


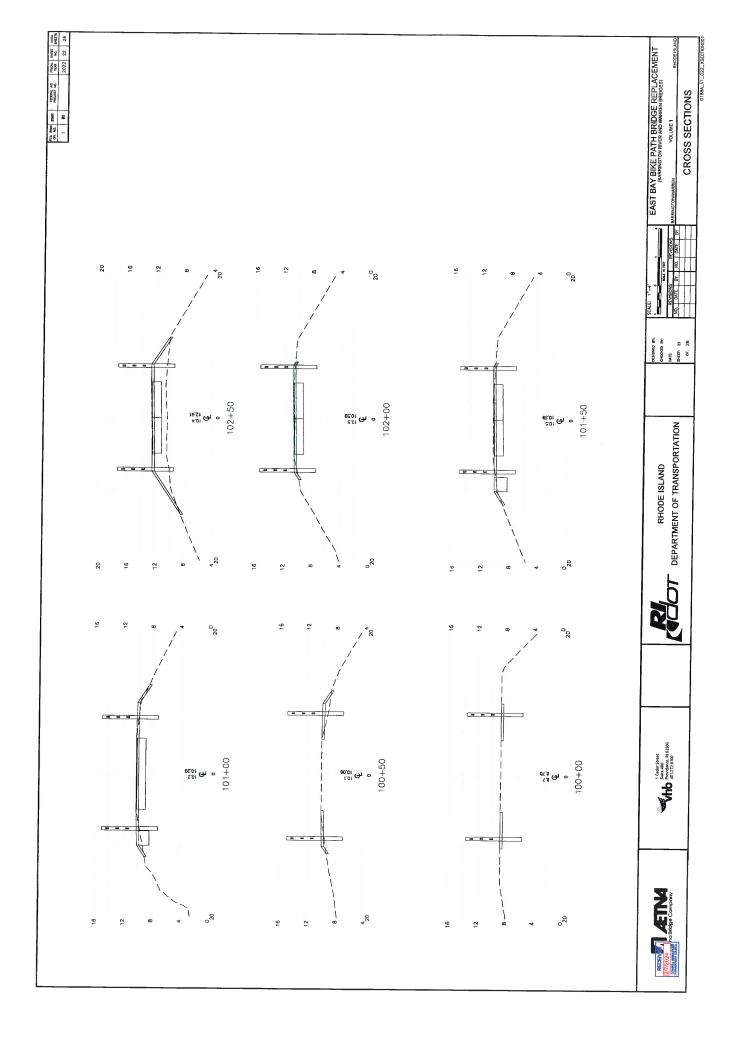


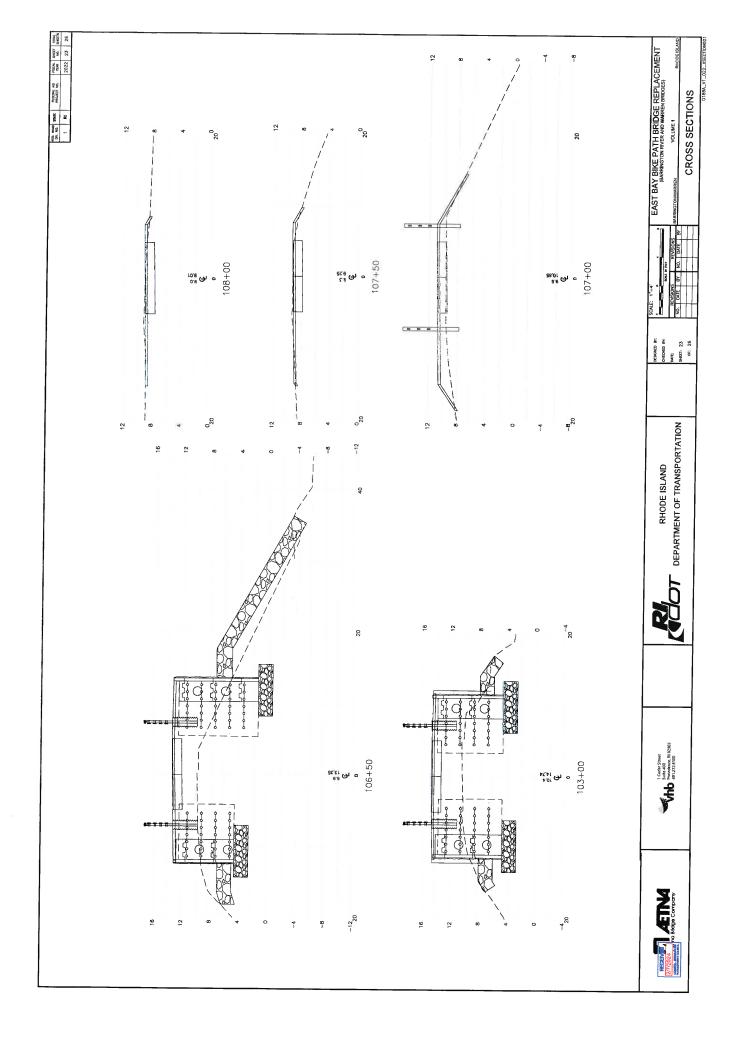


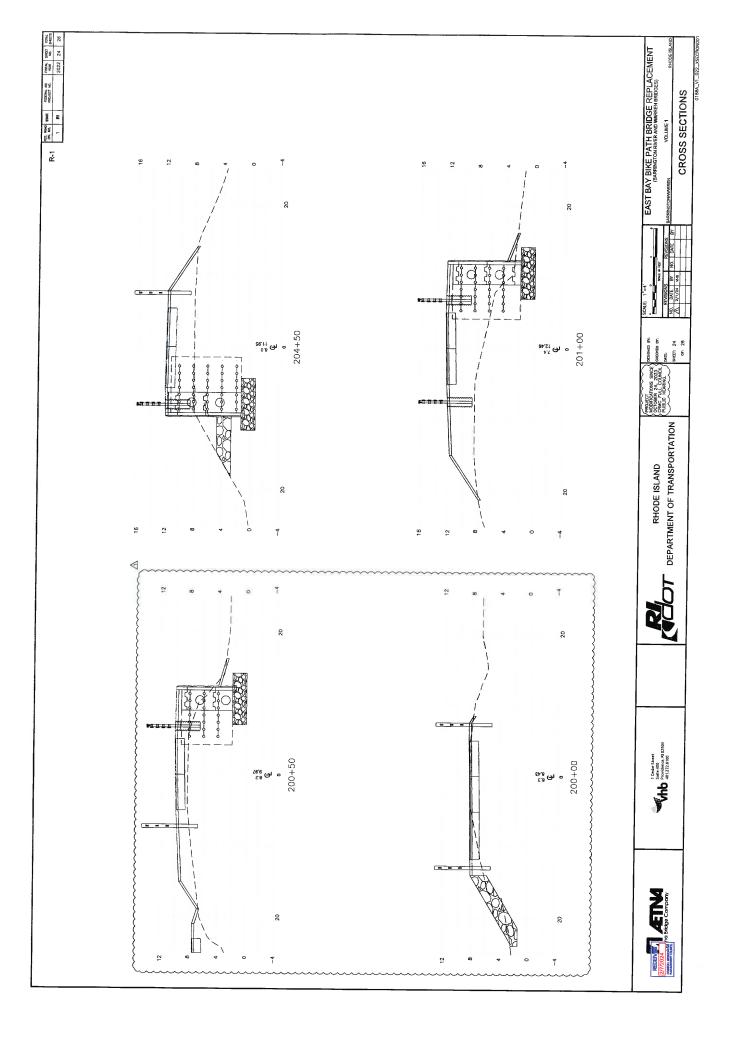


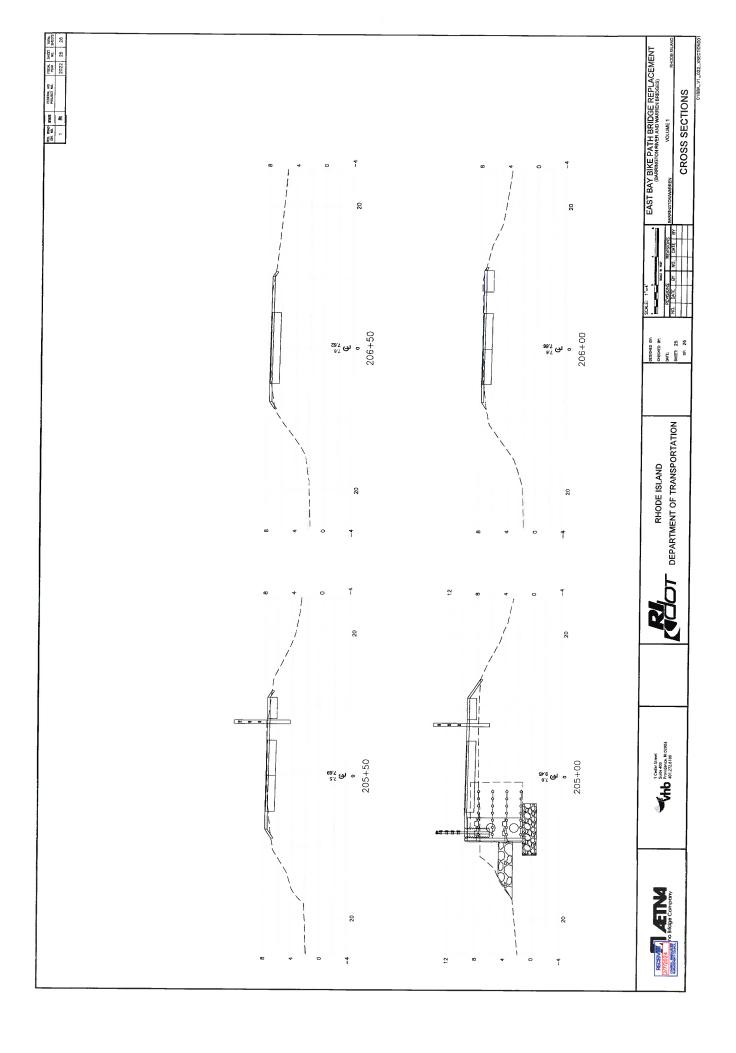


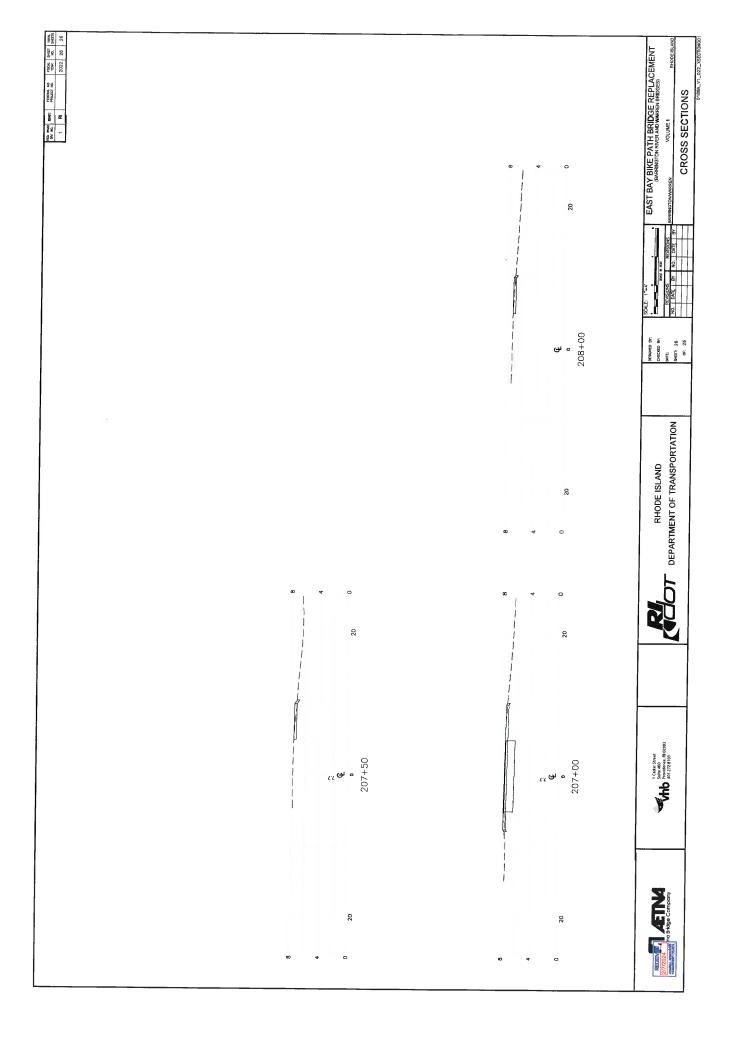


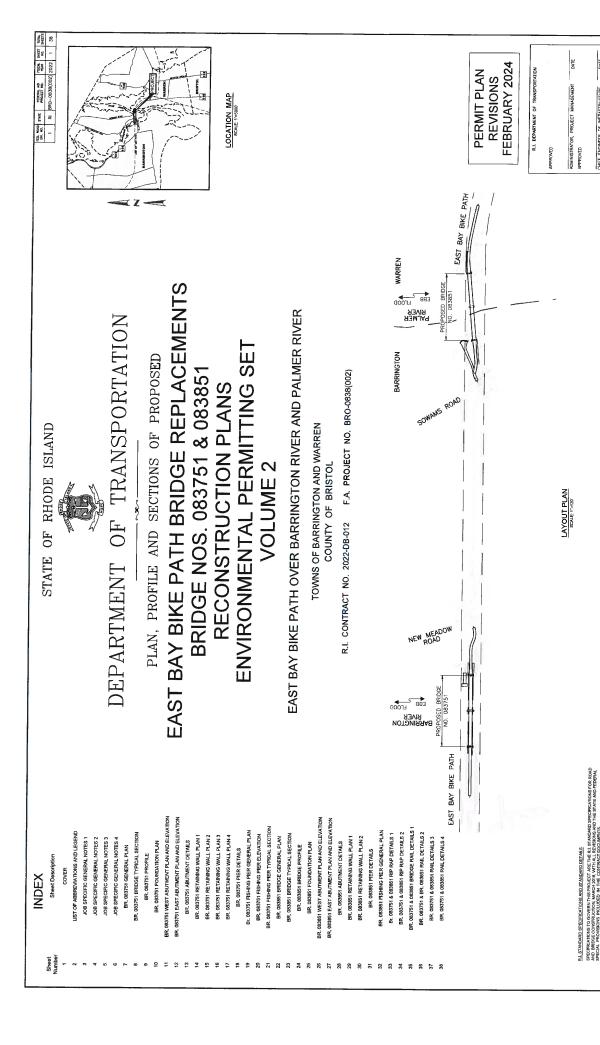












1884_V2_XXX_BCOV DATE

WISION ADMINISTRATOR

38

Contract Number 2022-DB-012

Number of Sheet

1 Cedar Street Suits 400 Providence, RI 02903 401.272.8100

BASE OF LEVELS NAVD 88 NAD 83 (2011)

Aelina Bridge Company

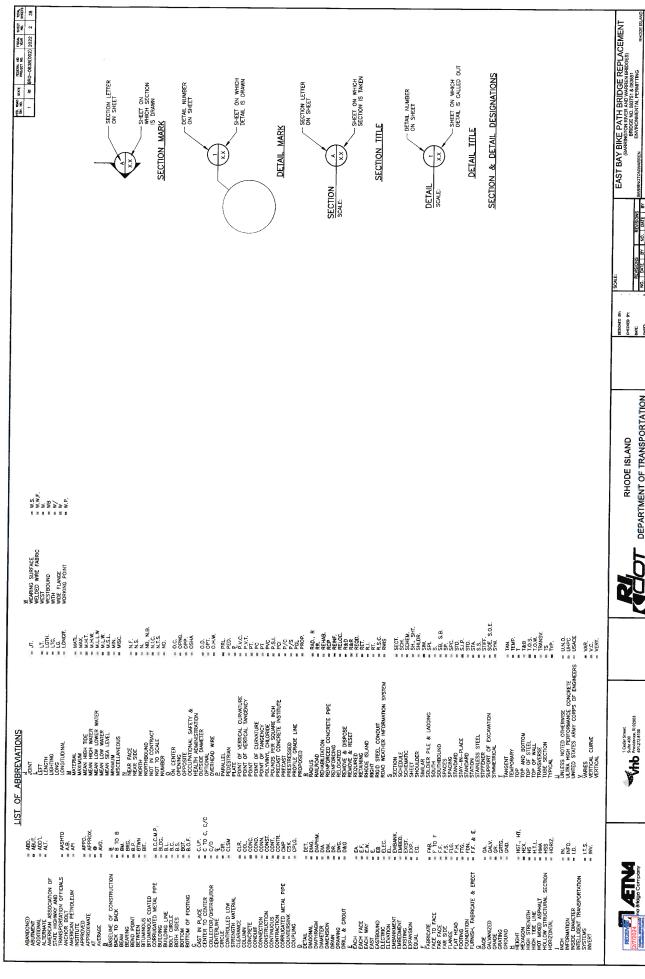
STANDARD DETAILS FOR THIS PROJECT ARER.I. STANDARD DETAILS, 1998 EDYTION, WITH ALL REVISIONS.

RECEIVED 2/7/2024

Total Sheets

DATE

CHIEF ENGINEER OF INFRASTRUCTURE



1 Cedar Street Suite 400 Providence, RI 02903 401-272.8100

RHODE ISLAND

RHODE ISLAND

DEPARTMENT OF TRANSPORTATION

DESIGNED BY: CHECKED BY: DATE: SHEET: OF:

S REVISIONS BA

EAST BAY BIKE PATH BRIDGE REPLACEMENT (BARRINGTON RUPE AND WAREEN BRIDGES) BRINGTON NO STORY & LOSSIS BRINGTON MEDITAL TEAMTING LIST OF ABBREVIATIONS AND LEGEND

SENERAL NOTES:

- 1. ALL CONSTRUCTION INDICATED ON THESE PLANS SHALL BE IN ACCORDANCE WITH:
- THE 2004 EDITION (AMENIED MARCH 2018) OF. AND SUPPLEMENTS TO, THE RHODE ISLAND DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION (RI STANDARD
- THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS (AASHTO) LIFD BRIDGE CONSTRUCTION SPECIFICATIONS, 4THEDITION, 2017, INCLUDING THE LATEST INTERM REVISIONS.
 - THE SPECIFICATIONS ACCOMPANYING THESE PLANS.
- IN CASE OF CONFLICT BETWEEN THE PLANS, SPECIFICATIONS OR MANUAL LISTED ABOVE, THE SPECIAL PROVISIONS OF THE SPECIFICATIONS ACCOMPANYING THESE PLANS SHALL GOVERN
- ALE ELEVATIONS ARE REFERENCED TO THE NATIONAL GEODETIC VERTICAL DATUM OF NAVD 88.
- 3. COORDINATES USED ON THESE PLANS ARE BASED ON THE STATEWIDE COORDINATE SYSTEM, THE NORTH AMERICAN DATUM OF 1983 (NAD 83 \not 2011).
- DMENSIONS, STATIONS, AND ELEVATIONS ARE SHOWN TO THE NEAREST ONE—HUNDREDTH OF A FOOT ONE OVER-SISTEMENT OF AN INCH. EXCEPT STRUCTURAL STEEL DMENSIONS WHICH ARE TO THE NEAREST ONE—STATEMENT OF AN INCH.
- ALL ANGLES ARE SHOWN TO THE NEAREST SECOND.
- TOPOGRAPHIC CONDITIONS WERE OBTAINED FROM AERIAL, PHOTOGRAMMETRY, ACCURACY OF VERTICAL TOPOGRAPHY IS WITHIN ONE—HALF OF A FOOT.
- 7. FOR BENCH MARKS AND TIES SEE HIGHWAY LOCATION PLANS.
- IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO FIELD VERIFY ALL ELEWITONS, DIMENSONS, SHALLS, AND LAYOUTS AS SHOWN ON THESE PLANS. THIS PROPER FIG. TO SEFECALLY PERTINENT FOR PRE-FABRICATED STRUCTURAL ITEMS AND WORK IN THE VICINITY OF UTILITIES.
- ETC. TEMPORARY PROTECTIVE SHELDING:
 DEBRIS SHELDS SALL DE PROVIDED AND INSTALLED TO PROTECT MOTORISTS, WATER WAYS, FROM ARY DEMOLITION OR CONSTRUCTION DEBRIS.
- 10. EXISTING DETAILS, DIMENSIONS AND ELEVATIONS PROVIDED IN THIS PLAN SET HAVE BEEN OBTAINED FROM THE ORIGINAL DRAWINGS AND SURVEY AND ARE NOT GUARANTEED.
- FIELD CONDITIONS WAY EXIST WINCH PRIVATE FOUR THE THEOLOGICAL AND THEOSPETINGL DIMENSIONS SHOWN ON THE PLANS. THE CONTRACTOR SHALL TAKE ALL FELD MENDABERIOR RECESSIONS ASSURE PROPER FIT OF THE FINISHED WORK AND SHALL ASSUME FULL RESPONSIBILITY FOR HISTORY AND SHALL ASSUME FULL RESPONSIBILITY.
- THE CONTRACTOR SHALL TAKE THE PROPER PRECAUTION TO ENSURE THE STABILITY OF ALL STRUCTURAL ELEMENTS DURING ALL PHASED CONSTRUCTION UNTIL THE TOTAL STRUCTURE IS IN PLACE.
- THE CONTRACTOR SHALL CALL DIG SAFE AT LEAST 72 HOURS PRIOR TO STARTING THE WORK TO VERIFY LOCATIONS OF EXISTING UTILITIES.
 - 14. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL COORDINATION WITH UTILITY OWNERS.
- ALL FOOTINGS SHALL BE APPROVED BY THE ENGINEER AS TO DIMENSIONS, ELEVATIONS, AND SUITABILITY OF FOUNDATION MATERIAL BEFORE THE PLACING OF CONCRETE.
- LE WORKING POINTS ARE SHOWN AT THE CENTERLINES OF BEARINGS OF ABUTMENTS, UNLESS OTHERWISE NOTED.
- - 17. ALL ABUTMENTS AND WALLS ARE DRAWN LOOKING AT THE EXPOSED FACES.
- THE EXISTING UTLITES SHOWN ON THE PLANS ARE APPROXIMATE AND WERE LOCATED LISING THE SET AMILDAGE INFORMATION. NO BUILDING SERVICE CONNECTIONS (ELECTRIC, TELEPHONE, CAS, WATER, SAMITEX, AND OTHERS) ARE SHOWN. THE CONTRACTOR IS TO ASSUME THAT SERVICES TO ALL BUILDINGS ARE PRESENT.
- 19. EOFT FEDERAL AND STATE LAW (RG CENTRAL LAW 39-1.2) REQUIRE NOTFOCHION OF APPROPRIATE GRANNING ENDORMER, BROKE BLOK FILLNG, GRANNING, LAWGSZAPING, OR OTHER PERSHA WOWN BENEFOR STATE OF THE CONTRACTOR'S NEW PROSENSE (NCLUDING THROUGH THROUGH TO NOTFOCK ALL THUM'S GREEN AND THROUGH TH

DESIGN SPECIFICATIONS

- ** THE AMSHTO LIRTO BRIDGE DESIGN SPECFICATIONS, 9TH EDITION, 2020, INCLIDING ALL INTERIM REVISIONS.

 ** THE AMSHTOL OHD GUIDE SPECIFICATIONS FOR THE DESIGN OF PEDESTRAN BRIDGES, 2009 MICLUDING ALL INTERIM REVISIONS.

 ** THE FAMOLICE STAND LEFT BRIDGE DESIGN MANUAL, 2007 EDITION INCLIDING, ALL ERISONOS.

 ** SLAMO LIPER BRIDGE DESIGN MANUAL DATE SOOT, STITLON 1 OF THE RHODE PERPARANT OF THE ROBE DESIGN MANUAL DATE SOOT.

 ** THE ZOOM EDITION (AMENDED MANUAL DATE) 2007.

 ** THE ROBE SECTION 1 OF THE RHODE ISLAND (RESPANDANT) SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION (RESISTANDAND SPECIFICATIONS).
- IN CASE OF CONFLICT, THE RHODE ISLAND LRFD BRIDGE DESIGN MANUAL SHALL GOVERN

LOAD MODIFIERS

- UNLESS NOTED OTHERWISE, THE LOAD MODIFIERS FOR THIS PROJECT ARE AS FOLLOWS:
- THE LOAD MODIFIER FOR DUCTIFYS SHALL BE TAKEN AS 10 FOR ALL UNIT STATES.

 THE LOAD MODIFIER FOR DUCTIFYS SHALL BE TAKEN AS 10 FOR ALL UNIT STATES.

 THE LOAD MODIFIER FOR OFFERTIONAL MICRITARES SHALL BE TAKEN AS 1.0 FOR ALL UNIT STATES.

LOAD FACTORS

ALL LOAD FACTORS SHALL BE IN ACCORDANCE WITH THE AMSHTO LRFD BRIDGE DESIGN SPECIFICATIONS, EXCEPT AS MODIFIED IN THE RHODE ISLAND BRIDGE DESIGN MANUAL.

- THE LOAD FACTOR FOR LIVE LOAD FOR THE EXTREME EVENT I LIMIT STATE SHALL BE TAKEN AS ZERO.
- THE LOAD PACTOR FOR LOAD FOR THE EXTREME ENEMY I AND EXTREME ENEMY I LIMIT STATE SHALL BE TAKEN AS 10.

LIVE LOADS

THE DESIGN VEHICULAR LIVE LOAD SHALL BE THE H15-44 DESIGNATION ADJUSTED FOR THE DYNAMIC LOAD ALLOWANCE, MULTIPLE PRESENCE FACTOR AND PER RI TAC 0347.

WIND LOADING DESIGN DATA

THE WIND LOADING DESIGN SHALL BE IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, THE RHODE ISLAND LRFD BRIDGE DESIGN MANUAL, AND AS MODIFIED HEREIN.

- EXCEPT DURING CONSTRUCTION, THE DESIGN WIND PRESSURE IS BASED ON A DESIGN WIND SPEED OF 140 MPH.
 - THE DESIGN WIND PRESSURES DURING CONSTRUCTION SHALL BE AS SPECIFIED UNDER THE NOTES TITLED "GENERAL NOTES REGARDING TEMPORARY CONSTRUCTION CONDITIONS".

TRAFFIC DATA

THERMAL DESIGN FORCE DATA

NUNIONE TRUPERATURE EFFECTS HAVE BEEN TAKEN WITO CONSIDERATION IN ACCORDANCE WITH THE PROCEDURE B OF THE AMBHOUND DESIGN TEMPERATURE SHALL BE —10 DEGREES F, AND THE MANNOW TRAFFRANCHE SHALL BE FIOS DEGREES F,

SEISMIC DESIGN DATA

PER RIDOT LRFD BRIDGE MANUAL AND AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, EAST BAY BIKE PATH BRIDGES 083751 & 083851 SHALL MEET SEISMC ZONE 1 DESIGN CRITERIA AND RFP ADDENDUM 3 GRITERA.

ALL REFERENCES IN THESE GENERAL NOTE SHEETS AND THROUGHOUT THE CONTRACT DRAWINGS TO THE RHODE ISLAND STANDARD SPECIFICATIONS FOR ROAD AND BRIOGE CONSTRUCTION SHALL EXCLUDE THE METHOD OF MEASURENT SECTION AND BASIS OF PAYMENT SECTION FOR ITEMS PAID FOR BY THE LUMP SUM ITEMS IN THIS CONTRACT.

FOUNDATION DESIGN DATA:

 TRD, IRAD
 STATE
 TEXTENA, AND
 FISSOL
 SHLETT
 TOTAL

 DAV, NO.
 TATAL
 FIG. 1
 SHLETT
 TOTAL

 1
 RR
 BRO-0838(DOZ)
 ZOZZ
 3
 38

VARIOUS SHALLOW Ħ 5 SPREAD FOUNDATIONS:
THE FACTORED BEARING RESISTANCE POUNDATION TYPES ARE AS FOLLOWS:

		BEARING RESISTANCE (KSF)	NCE (KSF)
LOCATION	TYPE OF BEARING MATERIAL	STRENGTH LIMIT STATES SERVICE LIMIT (#=0.45) STATES (#=1.0'	SERVICE LIMIT STATES (#=1.0)
ABUTMENTS	UNDISTURBED NATURAL GRANULAR SOILS	7	3.2

DEEP FOUNDATIONS: THE FACTORED AXIAL AND UPLIFT RESISTANCES FOR THE VARIOUS DEEP FOUNDATION TYPES ARE AS FOLLOWS:

FACTORED AXIAL RESISTANCE (KIPS)	TYPE STRENGTH LIMIT EXTREME LIMIT STATES STATES	DMP 200 286	DMP 220 315	DMP 218
	LOCATION	BR. 083751 PIER	BR. 083851 PIER	BR. 083751

DMP DMP DMP			RESISTANCE (KIPS)	C (KIPS)
DMP 123 DMP 127 DMP 23	LOCATION	TYPE	STRENGTH LIMIT STATES	EXTREME LIMIT STATES
DMP 127	751 PIER		123	141
23	BR. 083851 PIER		127	145
	383751 FMENT	DMP	23	26.5

- THE FACTORED DESIGN AXIAL RESISTANCE AT EACH LOCATION IS THE LESSER VALUE OF THE FACTORED GEOTECHNICAL AND THE FACTORED STRUCTURAL RESISTANCES INDICATED.
- THE FACTORED GEOTECHNICAL AXIA, RESISTANCE FOR THE STRENGTH LIMIT STATE IS BASED ON THE NOMINAL AXIAL RESISTANCE AS DETERAINED USING A NOMINAL BOND RESISTANCE OF 3.6 KSF AND A RESISTANCE FACTOR OF 0.7 (FOR COMPRESSION LODAINS).
- THE FACTORED GEOTECHNICAL AXIAL RESISTANCE FOR THE EXTREME LIMIT STATE IS BASED ON THE NOMINAL AXIAL RESISTANCE AS DETERMINED USING A NOMINAL BOND RESISTANCE OF 3.6 KSF AND A RESISTANCE FACTOR OF 1.0 (FOR COMPRESSION LOOMING).
- THE FACTORED GEOTECHNICAL AXIAL RESISTANCE FOR THE STRENGTH LIMIT STATE IS BASED ON THE NOMINAL AXIAL RESISTANCE AS DETERMINED USING A NOMINAL BOND RESISTANCE OF 3.6 KSF AND A RESISTANCE FACTOR OF 0.7 (FOR TENSION).
 - THE FACTORED GEDTECHNICAL AXIAL RESISTANCE FOR THE EXTREME LIMIT STATE IS BASED ON THE NOMINAL AXIAL RESISTANCE AS DETERMINED USING A NOMINAL BOND RESISTANCE OF 3.6 KSF AND A RESISTANCE FACTOR OF 0.8 (FOR TENSION).

NOTE:
MOULEA WILLS SHALL BE DESONED BY THE WALL SUPPLER. THE
COURTACTOR SHALL SUBMIT FOR REVIEW AND APPROVAL DESON
CALCULATIONS AND WORKING DRAWINGS SIGNED AND STAMFED BY A
PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF RHODE ISLAND.



3 è
Age Comp
RECEIV 277/2024

1 Cedar Street Suite 400 Providence, RI 02903 401:272.8100

OT DEPARTMENT OF TRANSPORTATION RHODE ISLAND

EAST BAY BIKE PATH BRIDGE REPLACEMENT
(BARRIOTON RIVER NO WARREN BRIDGES)
(BARRIOTON RIVER AND WARREN BRIDGES)
(BARRIOTON RIVER BRIDGES)
(BARRIOTON RIVER BRIDGES) JOB SPECIFIC GENERAL NOTES 1 REVISIONS REVISIONS B. NO. DATE BY NO. DATE BY NO. DESIGNED BY: CHECKED BY: DATE: SHETT: OF:

- STRUCTURAL STEEL:

 ASSHTO DESIGNATION M 270, GRADE 50, GALVANIZED

 IRUSS BRIDGE STEEL REFER TO CODE 801,9901 PREFABRICATED MODULAR BRIDGE FOR MATERIAL.
- API NB0 CASING

REINFORCING STEEL:

AASHTO DESIGNATION M 31, GRADE 60

- GALVANIZED COATING ASTM A767, CLASS I
- ASTM A615 GRADE 75 OR ASTM 722 GRADE 150 FOR MICRO-PILES

TREATED TIMBER

SOUTHERN PINE, GRADE NO. 1 OR BETTER 2x4 THRU 2x10, 4x4, 4x6 SHALL BE

HARDWARE AND FASTENERS:

CARRIAGE BOLTS, THREADED ROADS, LAG SCREWS HEX BOLTS

OGEE WASHERS

ASTM A307 GRADE A ASTM A307 GRADE A

COMPOSITE TOP RAIL:

COMPOSITE TOP RAIL BOARDS SHALL BE CONSTRUCTED, WANNEACTURED, AND
 FABRICACTED IN ACCORDANIC WITH ASSIV DYOS2—64 STAMBARD SPECIACIONIONS. BOARDS
SHALL BE SOLD COMPOSITE AND CAPPED. COLOR OF TOP PAIL SHALL BE GRAY.

CONCRETE STRENGTHS:

CLASS HP %" F'C=5,000 PSI RETURN WALLS, BACKWALLS

ALL COMPONENTS GREATER THAN 3 FEET IN THICKNESS, INCLUDING, BUT NOT LIMITED TO: ABUTMENT STEMS, PIER CAPS CLASS MC 34" F'C=3,500 PSI @ 28 DAYS, F'C=5,000 PSI @ 56 DAYS

CLASS XX %" F'C=4,000 PSI FOOTINGS 4,000 PSI NEAT CEMENT GROUT

CLASS A CONCRETE N" F'C=3,000 PSI

PEDESTRIAN BRIDGE RAIL POSTS

CONCRETE NOTES:

- CLASSES OF CONCRETE SHALL BE HIGH PERFORMANCE CLASS HP, CLASS MC AND CLASS XX, AS DESCRIBED THE HIGHOET SURVAIN STRANGEM SPECIFICATIONS FOR ROLAND BRIDGE CONSTRUCTION AND THE SPECIAL PROVISIONS OF THE SPECIALCIONS. RETER TO THE "MATERIALS" NOTES FOR CLASSES OF CONCRETE SPECIFIED FOR WARROUS COMPONENTS.
 - STEP COMPACTOR MY, AT THE APPROVAL OF THE BNOMER, PROPOSE THE USE OF STEP CONSOLUDIVING CONCRETE TOR ANY CLASS OF TOWNER ON THE PROJECT. SECTION 606 SELECONSOLUDIVING CONCRETE (SOC), CONTINUE THE REQUIREMENTS FOR MODERNIA ALL CLASSES OF CONCRETE MY RESISON FOR SILF-CONSOLUDIVING APPLICATIONS. THE MAXIMAM WIREF-CERENT FAITO FOR SICE SHALL BE C.40.
- ALL PORTLAND CEMENT CONCRETE SHALL BE AIR-ENTRAINED.
- ALL REINFORMEN STEEL SHALL BE GALANIZED, ALL WIRE TIES AND MISCELLANEOUS GALL WARDEN EUED FOR PLACEMENT OF GALANIZED REINFORMOS SHALL ASD BE GALANIZED. GALANNIZED. CORMING TO FATH APPS CAUSES 1.
- ALL LAP SPLICES NOT SHOWN ON THE PLANS SHALL BE LAPPED IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS FOR CLASS B LAP SPLICES.

UNIESS OTHERWISE INDICATED ON THE PLANS, ALL MAIN REINFORCING BARS SHALL HAVE THE FOLLOWING MINIMUM COVER:

PED, ROAD STATE PEDGRAL AND PEDGLA SHEET TIGHTS SHEET TIGHTS SHEET SHEET TIGHTS SHEET SHEE

THE CONTRACTOR'S REINFORCING BAR FABRICATOR SHALL VERIFY THE CORRECTIVESS IN PREPARING HIS GOBER USTS AND BENDING DIAGRAMS. SHOP DRAWINGS FOR ALL REDINOCEMENT DETAILS AND SCHOULE SHALL BE SUBMITTED TO THE ENGINEER IN SUFFICIENT TIME TO PERMIT CAREFUL CHECKING.

REINFORCEMENT NOTE:

CONCRETE CAST AGAINST OR PERMANENTLY EXPOSED TO EARTH (FOOTINGS, ABUTMENT WALL FACES, AND BACKWALLS)

ALL OTHER BARS

COVER TO TIES AND STRRUPS MAY BE 0.5 INCH LESS THAN ABOVE VALUES SPECIFIED FOR MAIN REINFORCING, BUT IN NO CASE LESS THAN 1.5 INCHES.

WILES OFTERWISE NOTED ON THE PLANS, ALL ANCHOR BOLTS SHALL BE ASTM DESIGNATION IN THE PLANS OF SECURATION IN ACCORDANCE WITH ASSETT OF SECURATION IN ACCORDANCE WITH SECTION MICE. SPECIOSED SHORS SHALL BE ASSETT DESIGNATION IN ACCORDANCE WITH ASSETT OF SHALL BE CALVANIZED IN ACCORDANCE WITH ASSETT OF SHALL BE CALVANIZED IN ACCORDANCE WITH ASSETT OF SHALL BE CALVANIZED IN ACCORDANCE WITH

- ALL ANCHOR BOLTS SHALL BE SET BY TEMPLATES PRIOR TO PLACEMENT OF CONCRETE UNIESS OTHERWISE INDICATED ON THE PLANS OR AUTHORIZED BY THE ENGINEER.
- HORIZONTAL CONSTRUCTION JOINTS OTHER THAN THOSE SHOWN ON PLANS WILL NOT BE PERMITTED WITHOUT A WRITTEN REQUEST BY THE CONTRACTOR AND PRIOR AUTHORIZATION BY THE ENGINEER.
- NEES OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CONCRETE SURFACES VISIBLE IN ELECATION TO ONE TOOT) BELOW THALL REQUINE UINE, SHALL RECEIVE A CONCRETE SURFACE RUBBED FINISH IN ACCORDANCE WITH THE RI STANDARD SPECIFICATIONS.
- 11. THE ENTIRE TOPSIDE SURFACES OF ABUTMENT BEAM SEATS, AS WELL AS VERTICAL FACES OF BACKWILLS SHALL BE PROVIDED WITH A FILM-FORMING SEALER (M12.03.1) CONCRETE SUFACE TREATMENT—PROTECTIVE COATING IN ACCORDANCE WITH SECTION 820 OF THE RISTANDARD SPECFFICATIONS.
 - 12. ALL EXPOSED EDGES AND REENTRANT CORNERS NOT OTHERWISE DETAILED ON THE PLANS SHALL HAVE A MINIMUM 1/2" CHAMFER.
- 13. THE CONTRACTOR SHALL BE RESPONSBLE FOR PREVENTING CONCRETE STANS OR DISCOLORATIONS DURING CONSTRUCTION UNIT, SLICH TIME SA THE SURFACES ARE APPROVED AND ACCEPTED BY THE ENGINEER. ANY CONCRETE STANS OR DISCOLORATIONS COLORENG PRIOR TO ACCEPTIANCE OF THE SURFACES SHALL BE REMOVED BY THE CONTRACTOR.
 - ALL JOINT SEALANT SHALL BE POLYURETHAME, POLYURETHAME ELISTOMERIC, OR SULCONE SEALANT AS DESTIDANCED ON THE PLANS. THE COLOR OF THE LIOHT SEALANT, WHERE POLYURESCE, SHALL BE NUTRAY (LIGHT GRAY OR TAN), THE COLOR OF THE SEALANT, WHERE NOT EXPOSED, WILL BE AT THE DISCRETION OF THE CONTRACTOR.
- UNLESS OTHERWISE NOTED ON THE PLANS, JOINT FILLER IS TO BE A PREFORMED, NON-ESPANSINE, NON-ESPANSINE, NON-ESPANSINE, NON-ESPANSINE, NON-ESPANSINE, STANDAND SPECIFICATIONS, TRUBBLE TO THE RISTANDAND SPECIFICATIONS, TRUBBLE TO THE RISTANDAND SPECIFICATION S.
- 16 EMEDMENT LENGTHS FOR DRILLED AND GROUTED DOWELS SHALL BE IN ACCORDANCE WITH SECTION 819 OF THE RI STANDARD SPECIFICATIONS, UNLESS OTHERWISE INDICATED ON THE PLANS.
- IN ACCORDANCE WITH THE RESTAURANGE SECREPLORISE, ALL WITALL IEES, NOW-METALLIC CONSTRUCTOR IN AT 18 TO SECREPLORISE. TO A MILLSON IN AT 18 TO SECREPLORISE. TO AN ILLEST WAN INCHES BEAR HE BY OSCIOUS SIGNATE. OF HE CONCRETE SURFACE. SHARKE OF HE SURFACE OF HE CONCRETE SURFACE. SHARKE OF HE SHARKE OF HE CONCRETE SURFACE. SHARKE OF HE SHARKE OF HE CONCRETE SURFACE. SHARKE OF HE SHARKE NO EXPOSENT HE TIES WILL SHARKE HE REDUCINE NOT HE STREAM HE SHARKE NO DEMONSTRAINE HAN HE TIES WILL SHARKE SHOUGH WITO THE STREAM HE COSPORED HE FORMER, SHARKE SHARKE SHOUGH WITO THE STREAM HE SHARKE SHA
- HAND—HELD VIBRATORS SHALL BE EQUIPPED WITH RUBBER TIPPED HEADS WHEN USED TO CONSOLIDATE CONCRETE AROUND REINFORCEMENT AND EMBEDMENTS.
- 14 WATEN STOPS ARE REQUIRED FOR HORIZONTAL AND VERTICAL CONSTRUCTION JOHN'S IN A MILL SHARE STOPS SHALL BE INSTALLED WIN WILL SHARE STOPS SHALL BE INSTALLED OF THE PANS, AT THE LOCATIONS AS RECEIVED ABOVE AND AT LL COLTIONS TO DRIVED ON THE PANS, AT THE LOCATIONS AS DIRECTION OF THE FINGURER, ALL IN ACCORDANCE WITH SECTION 912 OF THE RI STANDARD SPECIFICATIONS.
- UNIESS OTHERWISE DIMENSIONED ON THE PLANS, ALL REINFORCEMENT BENDS SHOWN ARE STANDARD HOOKS.
- AL EMPOSED FACES ABUNEAUS FROM THE BRIDGE SEATS TO THE GROUND SUFFACE AND EMPOSED WALL SURFINES A CONCRETE USERGE. TREATMENT PROTECTIVE SEALER THAT SURFACE TREATMENT PROTECTIVE SEALER THAT CONCRETE THE OWN IN COLOR A CLEAR NON-SCHREICH, TYPE ANTI-CRAFFITH CONCRETE PROTECTIVE COMING.
- 22. ANY METALLIC ELEMENTS THAT ARE TO BE LEFT IN PLACE AND NOT STATED HEREIN SHALL BE GALVANIZOE. THIS INCLUDES, BUT IS NOT LIMITED TO REINFORDING STEEL, WIRE MISSH. SNAP TIES, METAL TIES, ANCHORAGES FOR FORM WORK, SUPPORTS FOR MASS CONCRETE COOLING PIPES, ETC.

EARLY RELEASE FOR CONSTRUCTION (ERC. 1) APRIL, 2023

RHODE ISLAND EAST BAY BIKE PATH BRIDGE REPLACEMENT
(BARRINGTON RIVER AND WARREN BRIDGES)
(BRIDGE NO. GRETTE & GREGUE)
(BRINGON BRINGTON REPORTING JOB SPECIFIC GENERAL NOTES 2 REVISIONS REVISIONS B

Shidge Company

1 Cedar Street
Suite 400
Providence, RI 02903
401.272.8100

DEPARTMENT OF TRANSPORTATION A COT

RHODE ISLAND

DESYGNED BY: CHECKED BY: DATE: SHEET: GF:

01884_V2_004_JSBRGN

PRD. IRON SMITT FEDGUAL NO. FESCH. SMITT TOTAL DA. NO. NO. SMETT SMETT 1 RR GNO-0838(DOZ) 2022 5 38

STRUCTURAL STEEL NOTES

ALL STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH CODE 801,9801 PREFABRICATED MODULAR BRIDGE AND SECTION 824 OF THE RI STANDARD SPECIFICATIONS AS APPLICABLE.

TIMBER CONSTRUCTION NOTES

- ALL TIMBER SHALL BE PRESSURE TREATED, WOOD SHALL BE PRESSURE TREATED IN ACCORDANCE WITH THE AMERICAN WOOD PROTECTION ASSOCIATION (AWPA).
- GROUP CONTICT POSTS SHALL BE TREATED WITH PRESERVATIVES TO THE REQUIREMENTS FOR GROUND CONTICT/FRESHWATER CENTERUL USE SERVICE CONDITIONS (UCAN) IN ACCORDANCE WITH AWPA STANDARD UT ON GC-ES EMILLATION REPORTS.
 - ALL BOLTS SHALL BE ASTM A307 OR AS ALTERNATE A3125, GRADE A325.
- ALL BOLTED CONNECTIONS SHALL INCLUDE WASHERS AT BOLT HEADS AND NUTS.
- 5. LAG SCREWS SHALL BE LOW CARBON STEEL, ASTM A307 OR BETTER.
- ALL BOLTS, WASHERS AND OTHER HARDWARE SHALL BE GALVANIZED IN ACCORDANCE WITH AASHTO M232.
- TIMBER WHICH AT THE DISCRETION OF THE ENGINEER IS SEVERELY WARPED, BOWED, SPLIT, OR SPLINTERED SHALL NOT BE USED.
- B. FALLS AND RAIL CAPS SHALL BE CONTINUOUS OVER 1 POST MINIMUM, BUTT JOINTS IN RAILS AND RAIL CAPS SHALL BE STAGGERED AS SHOWN ON SHEET 35 TYPICAL ELEVATION OF PEDESTRIAN BRIOGE RAIL.
- 9. ALL TIMBER PAILING COMPONENTS SHALL BE TREATED WITH PRESERVATIVES TO THE REQUIREMENTS FOR AN ABOVE GROUND. DEPOSE SERVICE CONDITION (UC3B) IN ACCORDANCE WITH AMPA STANDARD LOT RICCAES EXLUDITON REPORTS.
- 10. TREAT ALL CUT ENDS, HOLES, NOTCHES AND RECESSES WITH COPPER NAPHTHENATE PRESERVATIVE. 11. ALL TIMBER SIZES ARE NOMINAL DIMENSION LUMBER UNLESS OTHERWISE NOTED.
- 12. LUMBER SUPPLED SHALL MEET THE REQUIREMENTS OF "SECTION BOG OF THE STANDARD SPECIOCACIONS" AND "THE 2018 MYNOWL DESION SPECIFICATIONS FOR WOOD CONSTRUCTION." CONDITION AND TEEM STRUCTIONAL TIMBER AND LLIMBER IN ACCORDANCE WITH THE "2018 NATIONAL DESION SPECIFICATIONS FOR WOOD CONSTRUCTION".
 - 13. GALVANIZED CARRIAGE BOLTS TO BE USED FOR ALL RAILING/POST CONNECTIONS.

DEMOLITION NOTES

- DIMENSIONS ARE BASED ON ORIGINAL DESIGN DRAWINGS AND ARE FOR INFORMATION ONLY, THE CONTRACTOR SHALL VERIEY ALL DIMENSIONS IN THE FIELD PRIOR TO COMMENCEMENT OF CONSTRUCTION.
 - A ROTHER SHALL REAMN IN PLOC. SO AS NOT TO CAUSE ANY SHORELINE DISTURBANCES. ONCE THE EMPROPMAL PROMAL PROMAL ONG NATURATION OF THE NEW DRODGES HAS BEEN GRANTED, THE EXISTING ABUNENTS MAY BE DEBOLOSHED.
 - IF THE CONTRACTOR'S DEMOLITION OPERATIONS CAUSE ANY DAWAGE TO ACCESS ROUTES AND PROPERTIES ONTSIDE OF THE PROJECT WORK AREA, THE CONTRACTOR SHALL BE REQUIRED TO REPAIR THE AREA TO THE SATISFACTION OF THE EMORIER.
- ALL DEMOLTION MATERALS SHALL BE CONTANED, COLLECTED, AND LEGALLY DISPOSED, IF DEBRIS FALLS TO THE RIVER, THE CONTRACTOR SHALL IMMEDIATELY REMOVE THE DEBRIS FROM THE WATER.
- THUREN DIES SHALL BE REMOVED IN THEIR ENTIRETY. STEEL PILES, THE STONE PIER IN THE BERRINGTON RINER, AND ANTIMBER PILES THAT BREAK MUST BE TRIMNED A MINIMUM OF 2"-0" BELOW THE RIVERBED SUBSTRATE LINE.
 - ALL DEMOLITION MATERIALS SHALL BE TAKEN FROM THE SITE TO AN APPROVED DESTINATION AS THE WORK PROGRESSES.
- THE CONTRACTOR IS NOTIFED THAT THE EXCITING PAINT SYSTEM OF THE STEEL AND THE TIMBER WAY NOTING YOUSTANDERS, SUCH AS LEAD, CHPOMING, OR CREGARITE, WHICH ANY REQUIRE SPECIAL HANDLING AND MAY BE HAZARODIS WASTE WRER REMOVED, PROTECT PRESONS AND EMPROMENT SPECIAL SHOWNEN THE REMOVAL. OF THE EXISTING STEEL, IN ACCORDANCE WITH SECTION 826 OF THE STANDARD SPECIFICATION.
- THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER, IN WRITING, HIS PROPOSED METHOD OF DEMOLITION. DEMOLITION OPERATIONS SHALL NOT BEGIN LAYTL, HIS SHEARSON SHALL INGLUE THE ENGINEER. THIS SUBMISSION SHALL INGLUE THE EDMOLITION PLANS, EQUIPMENT, SEQUENCE AND METHOD THE CONTRACTOR PROPOSED TO USE, IN DETAIL.

THE DEMOLITION AND FALSEWORK SUBMITIALS MUST BE STAMPED BY A PROFESSIONAL ENGINEER RESERTED IN THE STATE OF RHODE (SAMP). THE UNENSHING OF DEMOLITION AND FALSEWORK SUBMITIALS AND PLANS SHALL NOT SERVE TO RELIVE THE CONTRACTOR OF ANY PART OF HEIGH PRESPONSIBILITY FOR THE SAFETY OF THE WORK OR FOR THE SLOCKESSFUL COMPLETION OF THE WORK.





RHODE ISLAND

RECOT DEPARTMENT OF TRANSPORTATION

DESCANTO BY: CHECKED BY: DATE: SMEET: OF:

EAST BAY BIKE PATH BRIDGE REPLACEMENT (BARBATCH RIFFS AND WARREN BRIDGES)
BRIDGENO, 80578 & 605810
BRINGNARREN EPRAITING
BRINGNARRING BRANCHAL PERMITING JOB SPECIFIC GENERAL NOTES 3 REVISIONS REVISIONS NO. DATE BY NO. DATE BY

RECEIVE DE STITION OF STITION OF

1 Cedar Street Suite 400 Providence, RI 02903 401.272.8100

GENERAL NOTES REGARDING TEMPORARY CONSTRUCTION CONDITIONS:

1. DESIGN WIND PRESSURES FOR CONSTRUCTION:

MINIMUM WIND PRESSURES TO BE USED BY THE CONTRACTOR FOR DESIGN DURING THE CONSTRUCTION CONTRACT (WITH THE EXCEPTION OF SIGNS) SHALL BE FROM THE FOLLOWING THABLE:

WIND PRESSURE (PSF)	33	37	41	44	47
		33,	50,	75'	100
		유	٥	ဥ	2
200	11,	5	9	ď	ď
HEIGHT ABOVE GROUND	UP TO 17	AND	AND	AND	AND
HEIG		17,	33,	20,	78,
		OVER 17' AND UP TO 33'	OVER 33' AND UP TO 50'	OVER 50' AND UP TO 75'	OVER 75' AND UP TO 100'

ABLE NOTES:

- A. APPLICATION OF THE TABULAR PRESSURE:
- BRIDGE CARPONENTS DURING CORPUSEUTION, FROM TO THE NISTALLATION OF THE PERMANENT BRAZING SYSTEMS, NOT INCLUDIO CRAME LUTINO.
 FALSE WORK, SHORING, AND SCHITCIDING AS DETRIED IN FIRM GUIDE DESIGN OF THILSED FAMES FOR STORY OF TOWERS, EXCLIDING 3-DIMENSIONAL, LATTICED OF THILSED FAMES OF TOWERS.
 TIEMPORARY SHIELDING.

WIND PRESSURES FOR ALL OTHER STRUCTURES SHALL BE CALCULATED BASED ON ASCE DESIGN LOADS ON STRUCTURES BUNING CONSTRUCTION, SE/ASCE 37-02 (ALL REFERENCES THE ASCE 7 IN THE SEL/ASCE 37-022 PUBLICATION, SHALL BE THE LATEST REPISION OF ASCE 7), THE EXPOSURE CATEGORY SHALL BE C.

- WHERE APPLICABLE HIGHER AMTRAK WIND REQUIREMENTS SHALL SUPERSEDE THESE REQUIREMENTS.
- FOR STRUCTURES SITUATED ABOVE LIVE INTERSTATE TRAFFIC, THE TABULAR VALUES SHALL BE INCREASED BY 5 PSF.
- ERECTION OF BRIDGE COMPONENTS:
- FOR THE ERECTION OF STRUCTURES, THE FOLLOWING SHALL APPLY:
- THE CONTRACTOR SHALL SUBMIT AN ERECTION PLAN THAT PROVIDES COMPLETE DETAILS OF THE PROCESS. INCLUDIONG, BIT NOT LUMETED 15. TREMOSERY SUPPORTS, SCHEDLLING AND OPERATIONS SCHEDLLING AND CREADED THE CASE OF LIFTING. THIS APPLIES OF STREADED STREADED TO STREADED STREADED THE CASES DIVING WARMS STREADED STREADED STREADED TO STREADED STR
 - A REGISTERED PROFESSIONAL ENGINEER, LICENSED IN THE STATE OF RHODE ISLAND, WILL BE REQUIRED TO STAMP THE CONTRACTOR'S ERECTION PLAN.
- THE CONTROCTORS PROFESSIVEL ENGINEER WILL BE REQUIRED TO INSPECT AND PROVIDE BELOW. THE STREETS APPROVED THE SPECIAL OF INSTALLORING WITH CONTROLLOR STREET WAS NO ON BELOW THE STREETS WE PROFESSIONAL ENGINEER WAS ALLOWED STAMP ALL CHANGES TO SUBJECT OF STREETS WAS AND PROPOSED CHANGES WAS THE SUBJECT OF REPORT OF REMOVED THE STREET OF RECTOR OF A PROPOSAL DEMONSTREE WAS THE SUBJECT OF RESPECT OF A PROPOSAL DEMONSTREE WAS THE SUBJECT OF SUBJECT OF STREETS WAS A PROPOSAL DEMONSTREE WAS THE SUBJECT OF SU
- A MANDATORY PRE-ERECTION CONFERENCE WILL BE HELD AT LEAST TWO WEEKS PRIOR TO THE STARET OF THE GRORET MISSLALLOWN DO BISCUSS. THE PLAN AND PROSEDURES, WORK SCHOLLS, CONTINGATORY SEATEN, REQUIRELENTS AND TARFOT CONTROL. THE CONTROLL THE CONTROLL THE CONTROLL THE WINES ARE PRESENTED WINES WILL BE RECLUIRED TO THE DESIGN WILL BE RECLUIRED TO THE DESIGN OF THE DESIGN PROJECT THE DESIGN PROJECT THE CONTROLLOR SECURED TO WILL BE SECURED TO WILL BE STARED WINES WITH THE THE DESIGN PROJECT THE CONTROLLOR OF THE CONTROLLOR SECTION PLAN SHOOT MAY ORDER THE CONTROLLOR TO MODIFY AND RESUBANT THE ERECTION PLAN TO THE ENOMERER FOR REVIEW AND PRESENCE THE CONTROLLOR TO MODIFY AND RESUBANT THE ERECTION PLAN TO THE ENOMERER FOR REVIEW AND PREPROME
 - THE CONTRACTOR WILL BE REQUIRED TO PERFORM DALY INSPECTIONS OF THE ERECTED GIRDERS UNTIL THE BRIDGE DECK IS COMPLETELY POURED.

CONSTRUCTION NOTES:

- THE CONTRACTOR IS RESPONSIBLE FOR THE IMPLEMENTATION, CONSTRUCTION, OPERATION AND SAFETY OF ALL EQUIPMENT AND PROCEDURES.
- THE CONTRACTOR SHALL SUBMIT WORKING DOCUMENTS SHOWING PROPOSED METHODS OF LIFTING, SEQUENCING OF LIFTING, LOCATOR OF CAMERS, CRAIE CAPACHTES, LOCATION OF THE LIFTING POINTS ON THE BRIDGE COMPONENTS, WEIGHTS OF THE COMPONENTS, LIFTING DEVICES AND LOAD DISTRIBUTION DEVICE DETAIL. THE METHOD AND ALL SUBMISSIONS SHALL BE PREPARED AND STAMPED BY A RHODE ISLAND REGISTERED PROFESSIONAL ENGINEER.
 - COORDINATE ALL CONSTRUCTION ACTIVITIES WITHIN THE WORKING AREA WITH RIDOT REGARDING UTILITIES, PROTECTION OF TRAFFIC AND SCHEDULE.
- 4. THE CONTRACTOR SHALL EXERCISE EXTREME CARE TO ANDID DAMAGE TO EXISTING SIRUCTURES. ALL STRUCTURES DAMAGED AS A RESULT OF THE CONTRACTOR'S OPERATION SHALL BE REPAIRED TO THE SATISFACTION OF THE ENGINEER.
 - ALL SHOP DRAWINGS SHALL BE SUBMITTED TO THE ENGINEER IN SUFFICIENT TIME TO PERMIT CAREFUL CHECKING AS NOT TO DELAY THE PROJECT.

ĸ,

- ALL RIGGING IS TO BE IN EXCELLENT WORKING CONDITION. UNLOADED CRANES ARE ALLOWED TO TRAVEL IN THE WORKING AREA,
- CRANE DELIVERY LOCATIONS MAY VARY AS LONG AS MAXIMUM CRANE RADIUS IS NOT EXCEEDED.
- TEMPORARY EXCANSION SUPPORT SYSTEM SHALL BE DESIGNED, FURNISHED AND INSTALLED IN THE COPITION AND SUBMITED TO THE ENGINEER FOR REVIEW AND APPROVAL PRIOR TO THE START OF ANY EXCANSION.
 - CONTRACTOR SHALL SECURE ALL WORK AREAS AT ALL TIMES TO PREVENT UNAUTHORIZED ACCESS. 10.
- 11. STOCKPILED SOIL SHALL BE NOT CLOSER THAN 30 FEET FROM PIERS, WALLS AND ABUTMENTS.

UTILITY NOTES:

- 1. EXISTING UTILITIES SHOWN ON THE PLANS ARE APPROXIMATE AND WERE LOCATED USING THE BEST ANALIABLE INFORMATION. NO BULLIDNG STRAVCE CONNECTIONS (ELECTRIC, TELEPHONE, GAS, WATTER, SANITHYR AND OTHERS) ARE SHOWN. THE CONTRACTOR IS TO ASSUME THAT SERVICES TO ALL BUILDINGS ARE PRESENT.
- BOTH FEDERAL AND STATE LAW (R. CENERAL LAW 39-1.2) REDUIRE NOTFICATION OF APPROPRIATE UTILITY COMPANEES BEFORE GENERAL, ERECURIES, BLISTING, BOADLOSHING, BORING, LANDSCAPING, OR OTHER EARTH MOWNG OFFERCHORS. IT IS THE OWNSTACTOR'S RESPONSIBILITY ON NOTITY ALL UTILITY COMPANEES (INCLUDING THROUGH THE "DIG SAFE PROCRAM,") TO RESURE THAT ALL UTILITIES, ORNERORS (INCLUDING SHOULD). UNDERSTAND THAT NOT ALL UTILITIES WHEN OR SICH WORKELEAFOR IS SHOW MORE SHORT OF SHOW SOME SHORT OF THE "DIG SAFE PROCRAM, ANY DAMAGE TO EXISTING UTILIES WHEN DIE THE REPUBLIES WHEN DIE THE THE REPUBLIES WHEN DIE THE REPUBLIES WHEN DIE THE REPUBLIES OF ALL OF FALLING OF FALLING THE STATE AND/OR THE MARCED UTILITY COMPANY) AT NO ADDITIONAL COST TO THE
- CONSTRUCTON EQUIPMENT OR PERSONNEL SHALL FOLLOW OSHA REGULATION IN REGARDS TO MINIMUM CLEARANCE TO ENERGIZED OVERHEAD LINES.
- 4. UNDEGROUND ULTU LINES MAY BE IN CONFLICT WITH REQUIRED TEUPORARY OR PERMANENT CONSTRUCTION, OTHER EQUINEDD TREASPACE TO BETTOOM THE REQUIRED TO RESTORATE TO BE RELOCATION FROM THE CONSTRUCTION, THESE ULTHINS MAY NEED TO BE RELOCATION FROM ON THE CONSTRUCTION PERSON AND THEN MOVED BUCK TO PERMANENT LOCATIONS. THE GATOL THE RESPONSIBILITY OF THE INDIVIDUAL UNLITY OF THE INDIVIDUAL UNLITY OF THE INDIVIDUAL UNLITY OF THE INDIVIDUAL UNLITY OWNER. HOWEVER, HOW TO PRESENT HE RESPONSIBILITY OF THE INDIVIDUAL UNLITY OWNER, HOW TO PAUSE HES COMPRISED TO CONDIMINE THE INDIVIDUAL UNLITY OWNER, AND TO PAUSE HES COMPRISED TO CONDIMINE THE INDIVIDUAL UNLITY OWNER, AND TO PAUSE HES COMPRISED TO MAKE AS FOLIDIED TO COMPANIE THE OFFICIAL WAS AND CORRESPONDENCE. THE OFFICIAL WAS AND CORRESPONDENCE. THE OFFICIAL WAS AND CORRESPONDENCE. THE MANDROLL UNLITY POLL AND CONDUIT LOCATIONS, CONTRACTOR SHALL ENGAGE IN THE HEESTSSANT COORDINATION AND THE CONDUIT LOCATIONS. OF THE MEDICATION WORK. THE CONDUIT LOCATIONS AND CORPUSED TO AND CONDUIT LOCATIONS. AND CORPUSED TO AND CONDUIT LOCATIONS AND CORPUSED AND CORPUSED AND CONDUIT LOCATIONS AND CORPUSED AND CORPUSED AND CONDUIT LOCATIONS AND CORPUSED AND CORP

TEMPORARY CONSTRUCTION STRUCTURAL STEEL NOTES:

 TEL BAND
 STAZE
 FEDGRAL ADD
 FESCAL
 SPEET
 TOTAL

 BAN HOL
 PROLET HOL<

- STRUCTURAL STEEL SHAPES AND PLATES SHALL CONFORM TO THE LATEST PROVISIONS OF THE ASTM DESIGNATION A 709 GRADE 36 OR AS DESIGNATED ON THE PLANS.
- HIGH STREEL BOLTS SHALL CONFORM TO ASTM DESIGNATION A F3125 GRADE A225. THE COMPACTOR SHALL REFER TO SECTION 28 Y CONNECTIONS USING HIGH STREAMTH BATENSTHE BOLTS OF THE STATE OF PRODE ISLAND STRANDARD SPECIFICATIONS FOR RECOM AND BRIDGE CONSTRUCTION (1997) FOR MATERIAL AND INSTALLATION REQUIREMENTS. ALL HIGH STREAMTH BOLTS SHALL BE 7/8° DIAMETER ON 15/16°.
 - MISHERS METING ASTM DESIGNATION F 436 ARE TO BE USED OVER ALL HOLES THAT ARE MORE THAN ** IN DAMETER GREATER THAN THE BOLT DAMETER AND UNDER ALL PARTS TURNED DURING ASSEMBLY.
- 4. WELDING SHALL BE IN ACCORDANCE WITH THE LATEST STRUCTURAL WELDING CODE ANSI/AASHTO/AWS D15-2015 (INCLUDING ALL INTERNS TO DATE) AND APPLICABLE SUPPLEMENTAL ANS PUBLICATIONS. ALL OPPO CONNECTIONS SHALL BE WELDED AND ALL PRLD CONNECTIONS SHALL BE BOLTED UNLESS OPPERWAS. NOTED.
 - WELDING ELECTRODES SHALL HAVE THE SAME CORROSION RESISTANCE AS THE BASE METAL.
 - NO SHOP FILLET WELD SHALL BE LESS THAN 3/16", UNLESS OTHERWISE SPECIFIED.
- PRIOR TO FABRICATION, ALL MATENIALS SHALL BE BLASTI-CLENED TO AT LEAST SSPC-SP6 TO REMOVE. LCL, DRT, GRANGES, MILL SCALE AND OTHER DELETEROUS WATENIALS FROM THE SURFACES OF THE STELL TO BE PRHEOCKED.
- WHEN STEEL DIE STAMPS ARE USED TO IDENTIFY PIECES AND MEMBERS, FABRICATORS SHALL UTILIZE LOW STRESS STAMPS.

DESIGN TIDAL INFORMATION

EL. 3,78	EL. 2.12	EL1.90	EL2.47	EL. 9.70
HIGH TIDE LINE (USACE HTL) =	R		-Oir	u
UNITED STATES ARMY CORPS ENGINEERS HIGH TIDE LINE (USACE HTL)	MEAN HIGH WATER (MHW)	MEAN LOW WATER (MLW)	MEAN LOW LOW WATER (MLLW)	100 YEAR FLOOD (BARRINGTON RIVER)

THE CONTRACTOR SHALL NOTE THAT HIGHER AND LOWER TIDES ARE POSSIBLE.

100 YEAR FLOOD (PALMER RIVER)

EL. 8.20





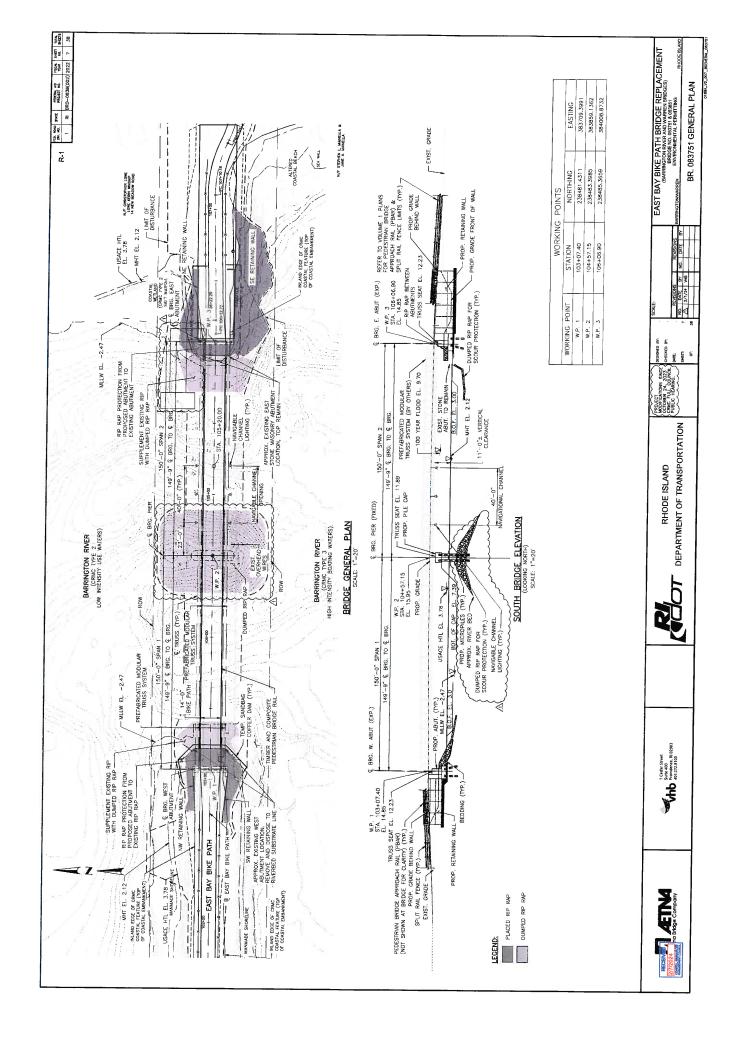
1 Cedar Street
Suite 400
Pravidence, RI 02503
401,272,8100

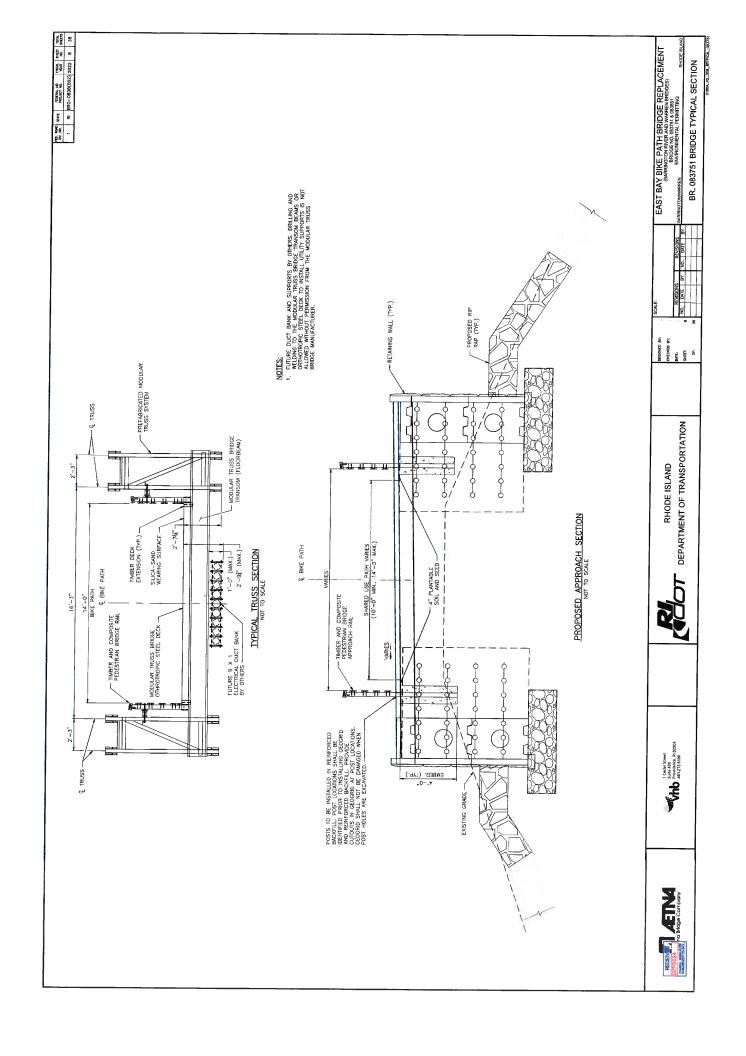


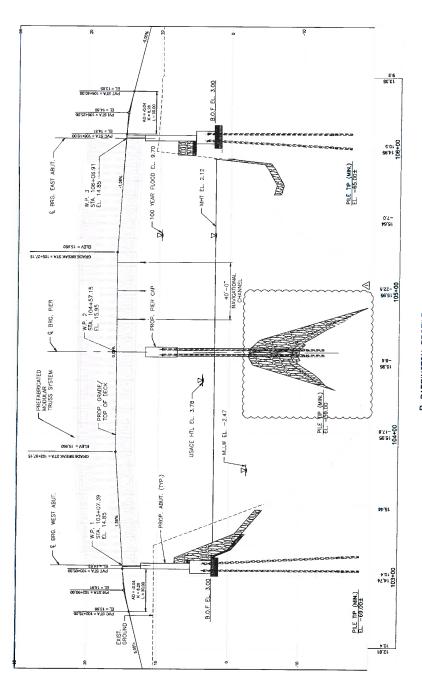
RHODE ISLAND

PEPARTMENT OF TRANSPORTATION

	1400	١	l	l	ļ	١	
DESIGNED BY:	{	ų.					EAST BAY BIKE PATH BRIDGE REPLACEMENT
CHECKED BY:							(BARRINGTON RIVER AND WARREN BRIDGES) BRIDGE NO. 083751 & 083851
DATE		REMISION	S	Ľ	REVISION	٥	BARRINGTONIAMODEN ENVIRONMENTAL PERMITTING
Saff.	ě	DATE	æ	ON.	DATE	à	RHODEISLAN
	j					L	
- Se						L	JOB SPECIFIC GENERAL NOTES 4
				l			







B BARRINGTON PROFILE SCALE HORIZONTAL: 1"=20' SCALE VERTICAL: 1"=4'

 $\frac{\mathsf{NOTE}_{:}}{\mathsf{REFER}}$ To Highway plans for additional information.

EAST BAY BIKE PATH BRIDGE REPLACEMENT
(BARRANCTOR INC. AND WARREN BROGES)
(BARRANGEN)
(BARRANGEN)
(BARRANGEN)
(BARRANGEN)
(BARRANGEN)
(BARRANGEN)
(BARRANGEN) PROJECT SINCE CHORD BY CONTROL SINCE CHORD BY CONTROL COUNCIL COUNCIL

DEPARTMENT OF TRANSPORTATION

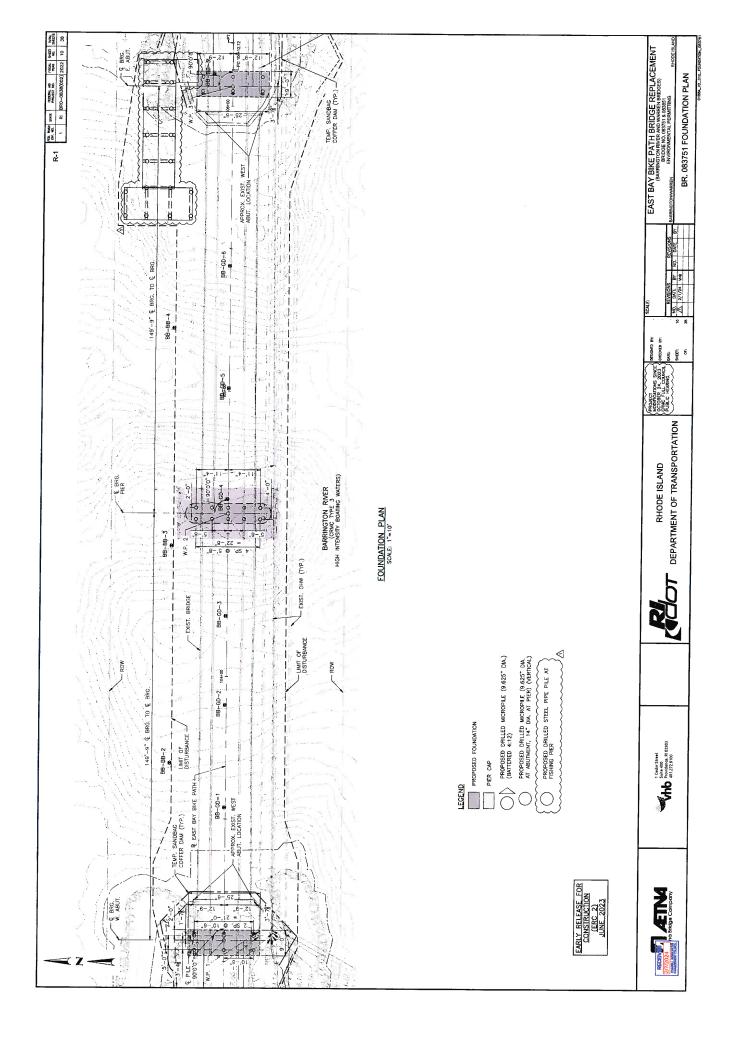
RHODE ISLAND

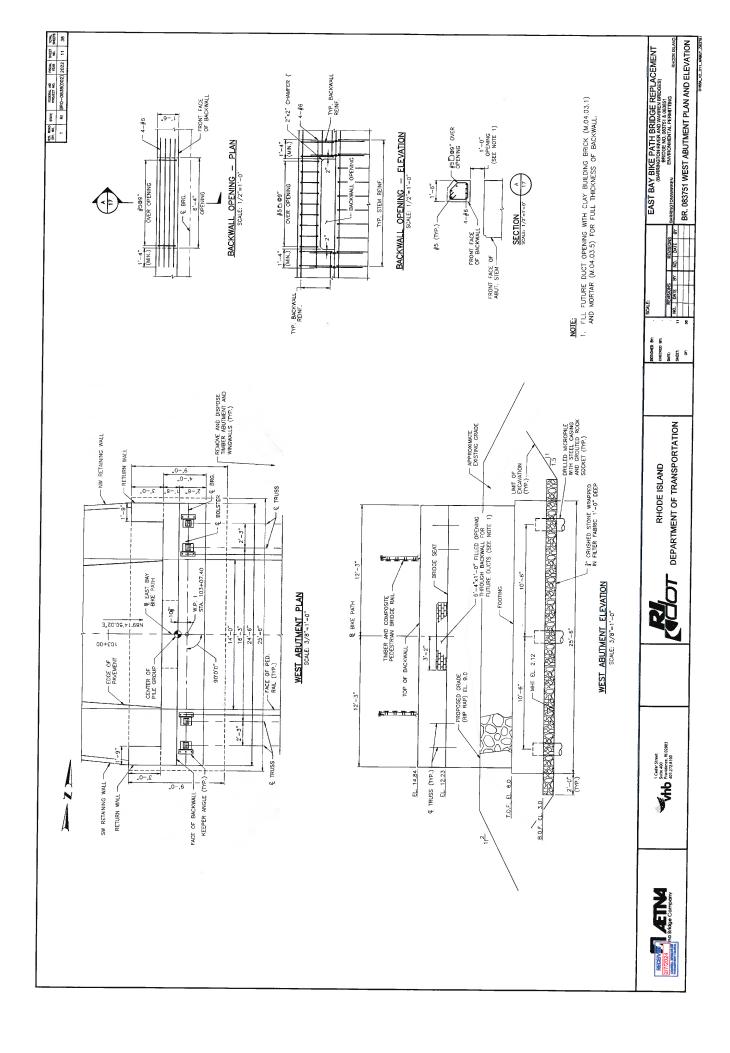
BR. 083751 PROFILE

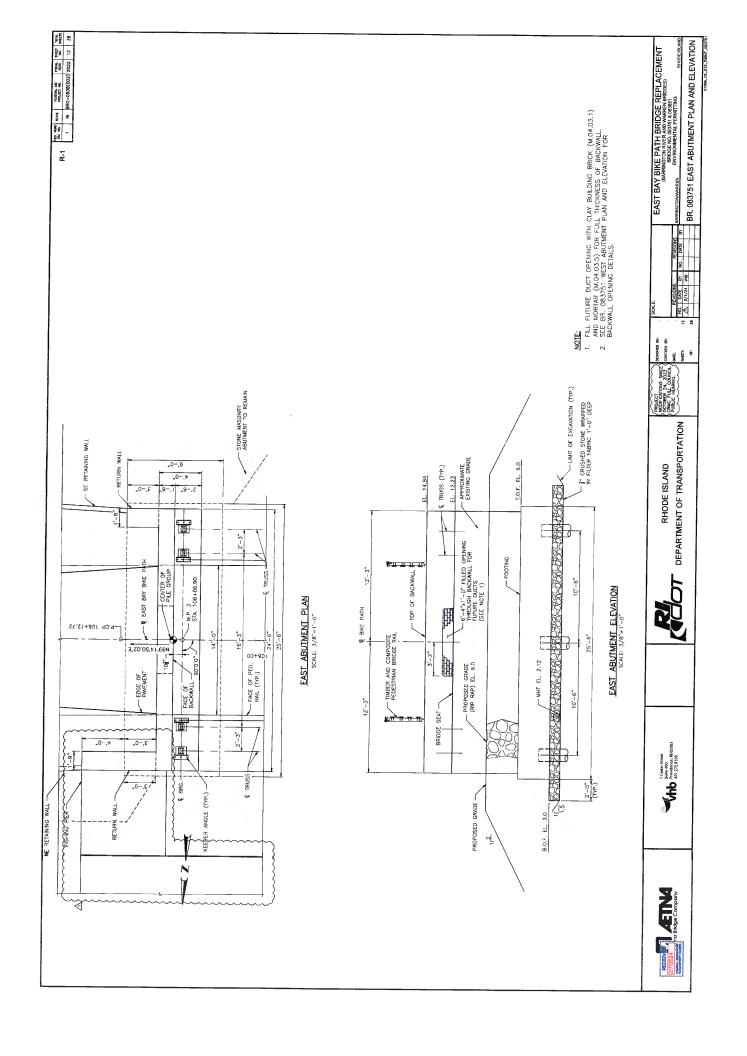


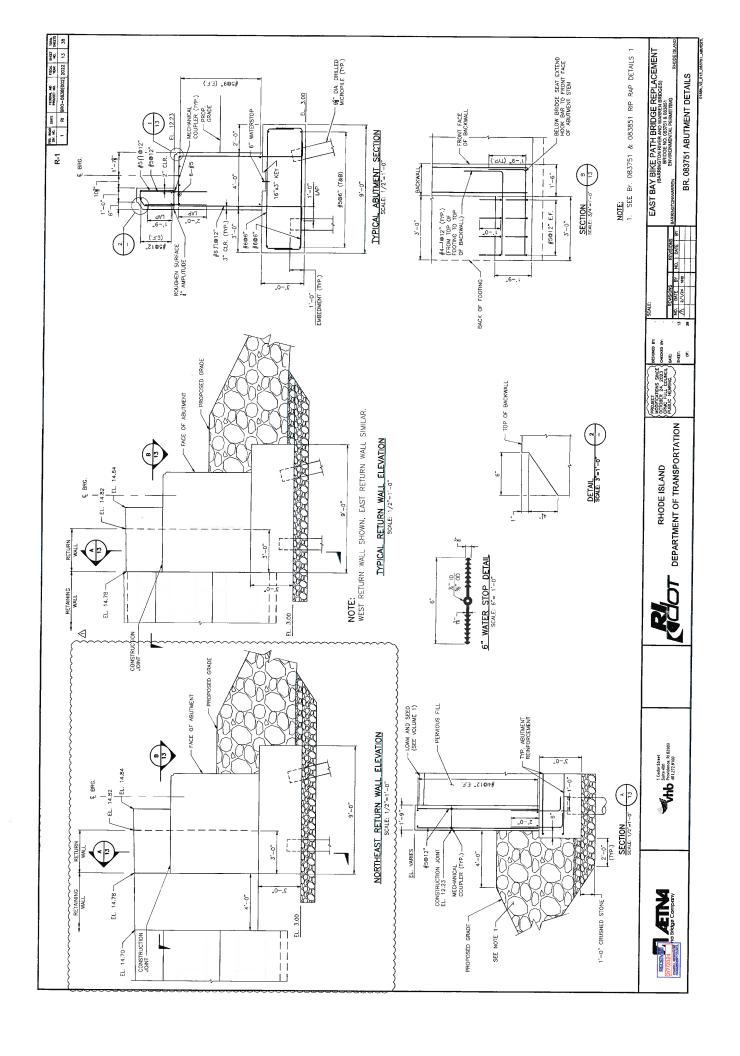
1 Cedar Street Suite 400 Providence, RI 02903 401 272.8 100

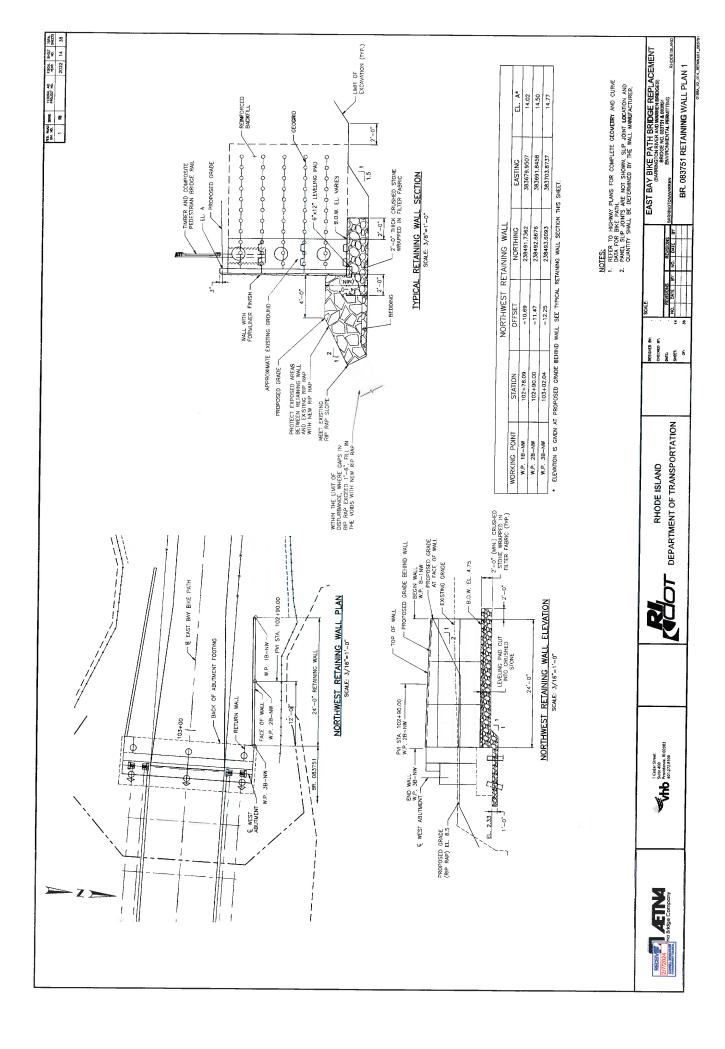
na Bridge Company	
PETEN II	











TD, NO. SIATE FEDERA, AND 1952M, SHEET TOTAL OF NO. 1952M, SHEET TOTAL SHEETS S EL. A* 13.65 14.51 14.77 EASTING 383672.3115 383692.1417 383704.1960 . ELEVATION IS GIVEN AT PROPOSED GRADE BEHIND WALL, SEE TYPICAL RETAINING WALL SECTION. 1. SEE BR. 083751 RETAINING WALL 1 SHEET FOR TYPICAL RETAINING WALL SECTION AND NOTES. NORTHING 238471.8537 238470.1482 SOUTHWEST RETAINING WALL 9.09 STATION 102+70.19 102+90.00 103+02.04 WORKING POINT
W.P. 18-SW
W.P. 28-SW
W.P. 38-SW NOTES: RIP RAP) EL. 9.00 -€ WEST ABUTMENT © WEST ABUTMENT BR. 083751 8 SOUTHWEST RETAINING WALL ELEVATION SCALE: 3/16"=1"-0" SOUTHWEST RETAINING WALL PLAN SCALE: 3/16"=1'-0" φ W.P. 3B+SW — END WALL W.P. 3B-SW — RETURN WALL --BACK OF ABUTMENT FOOTING -LIMIT OF DISTURBANCE 7 EL. 6.67 EXISTING GRADE 32 -0" RETAINING WAL SPLIT RAIL BRIDGE APPROACH RAIL W.P. 18-SW W.P. B-2SW — - B EAST BAY BIKE PATH COMPOST FILTER SOCK 12" DIAMETER 32-0" RETAIL PVI STA. 102+90.00 W.P. 2B-SW PROPOSED GRADE BEHIND WALL BRIDGE APPROACH RAIL - BEGIN WALL W.P. 1B-SW SPLIT RAIL FENCE 2'-0" (MIN.) CRUSHED STONE WRAPPED IN FILTER FABRIC (TYP.) TOP OF WALL -TEMPORARY LIMIT OF REGRADING — PROPOSED GRADE Z Z. -



RHODE ISLAND

RHODE ISLAND

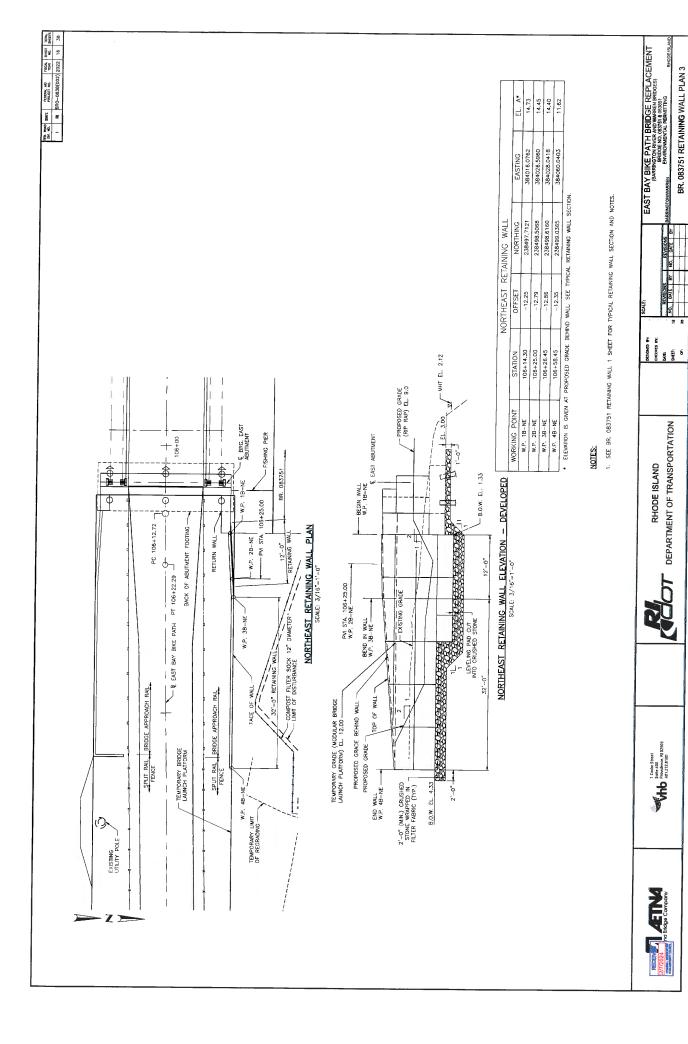
DEPARTMENT OF TRANSPORTATION

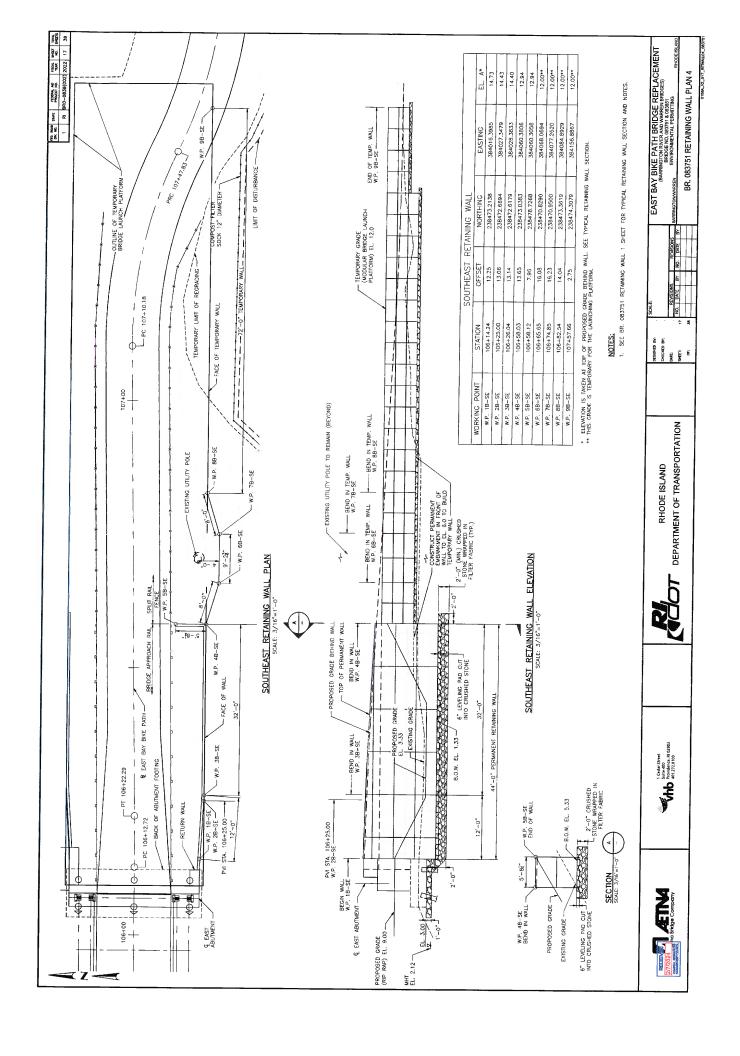
ľ		Γ	Š	I.		١	l	l	ſ	
	DESIGNED BY:									EAST BAY BIKE PATH BRIDGE REPLACEMENT
	CHECKED BY:									(BARRINGTON RIVER AND WARREN BRIDGES) BRIDGE NO. 083751 & 083851
	DATE		æ	EVISION	S.	Ц	Š	SIONS	Ι	
		-	NO.	DATE	B.	Ł	â	L J	à	RHODEISLAND
					L	L	H	H		
	ë	23				Ц	L	-		BK. 083751 RETAINING WALL PLAN 2
Ī		_				L	L	r		

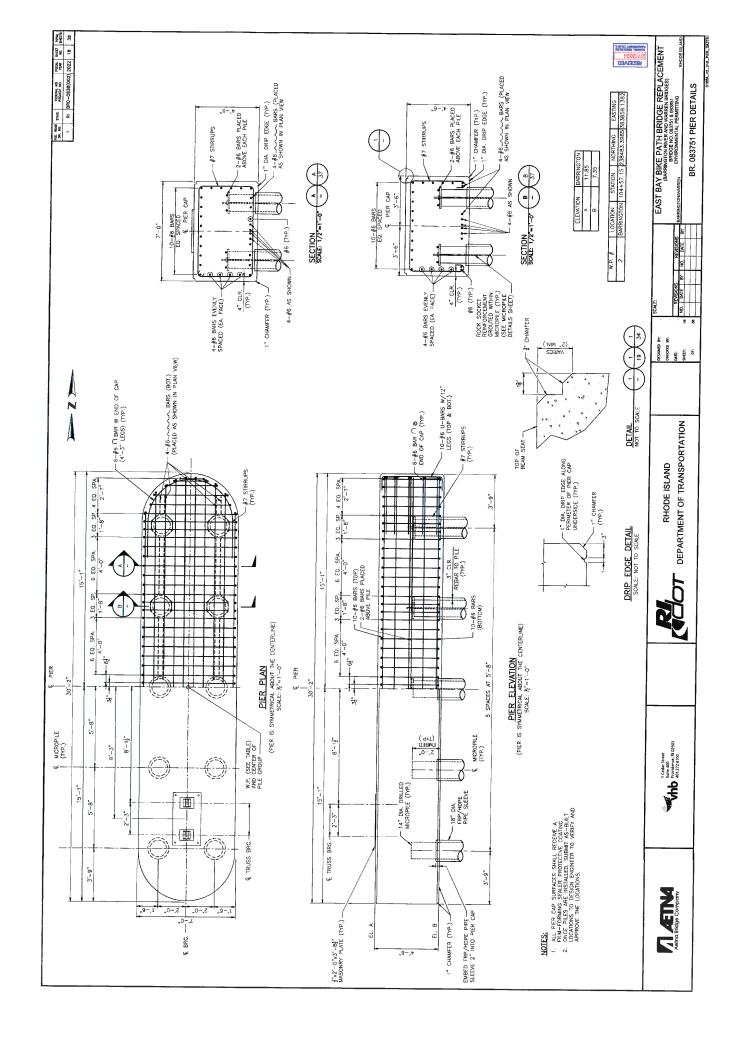
0188A_V2_015_RETWALLO2_083751

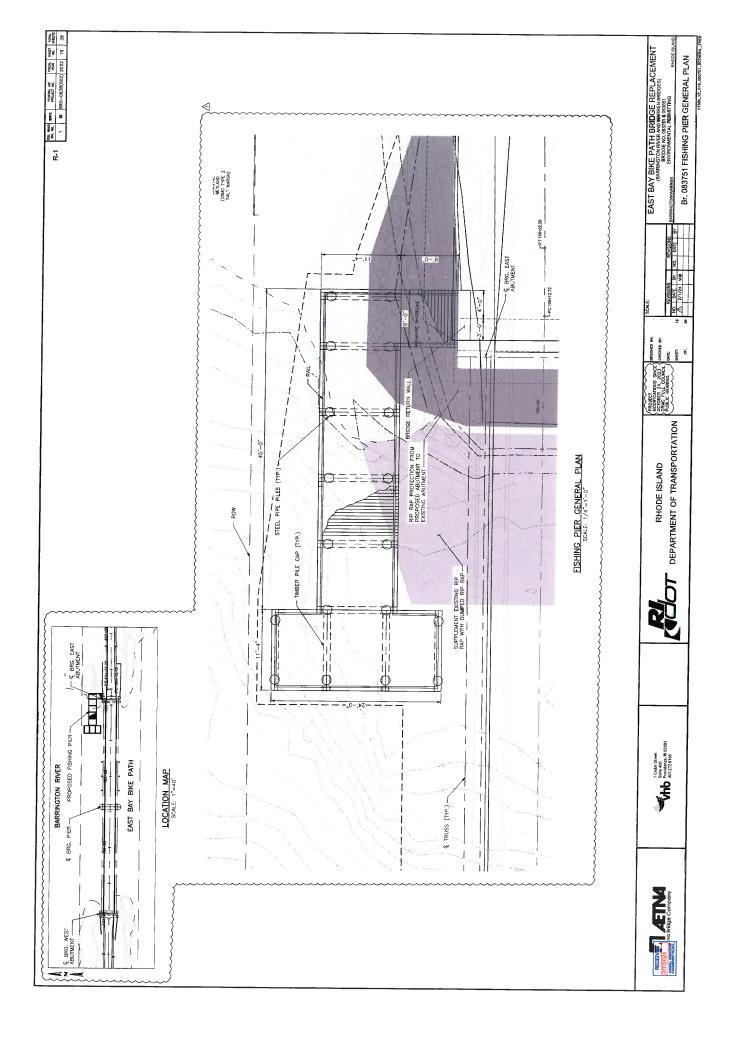


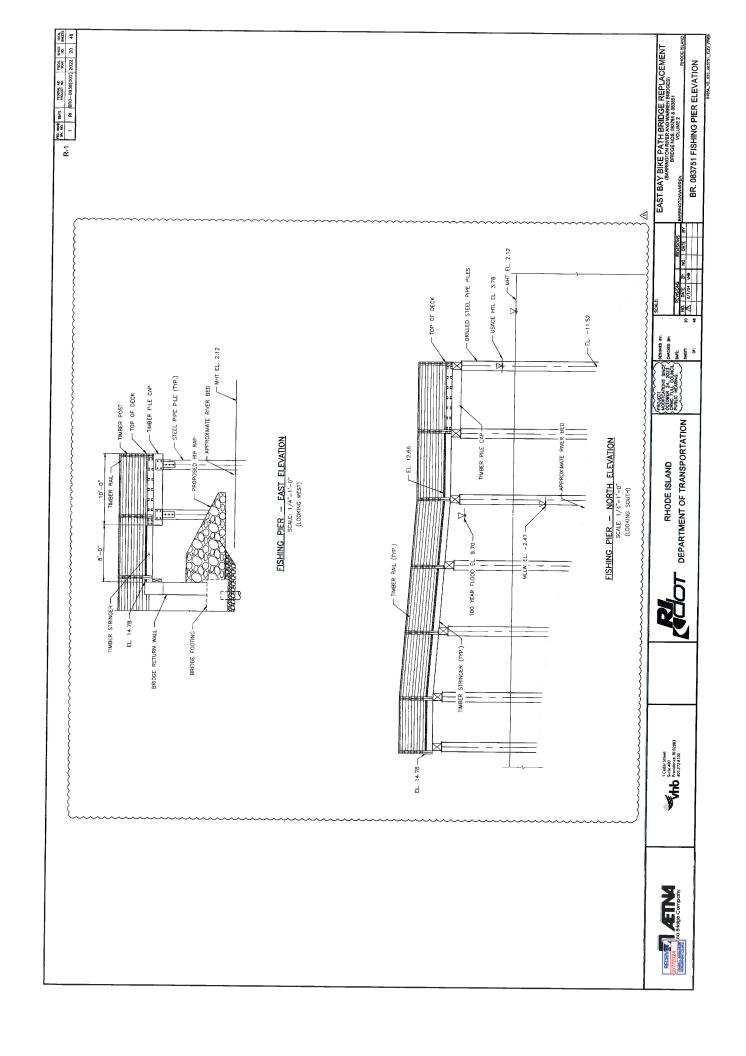
1 Cedar Street Suite 400 Providenca, RI 02903 401,272,8100

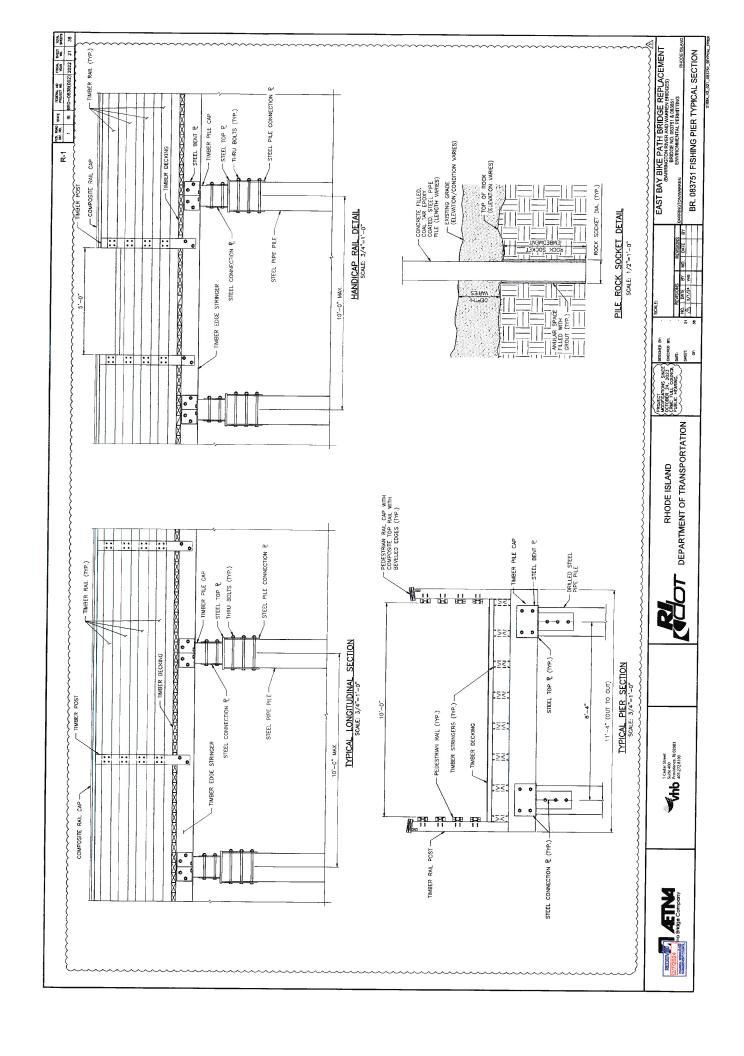


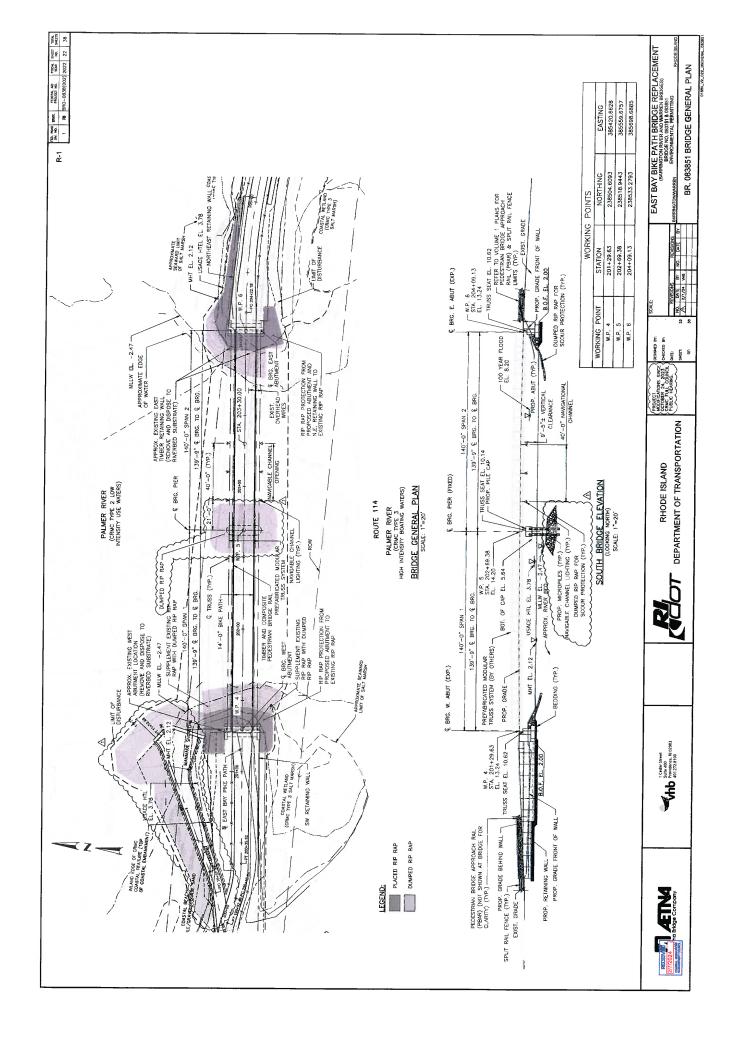


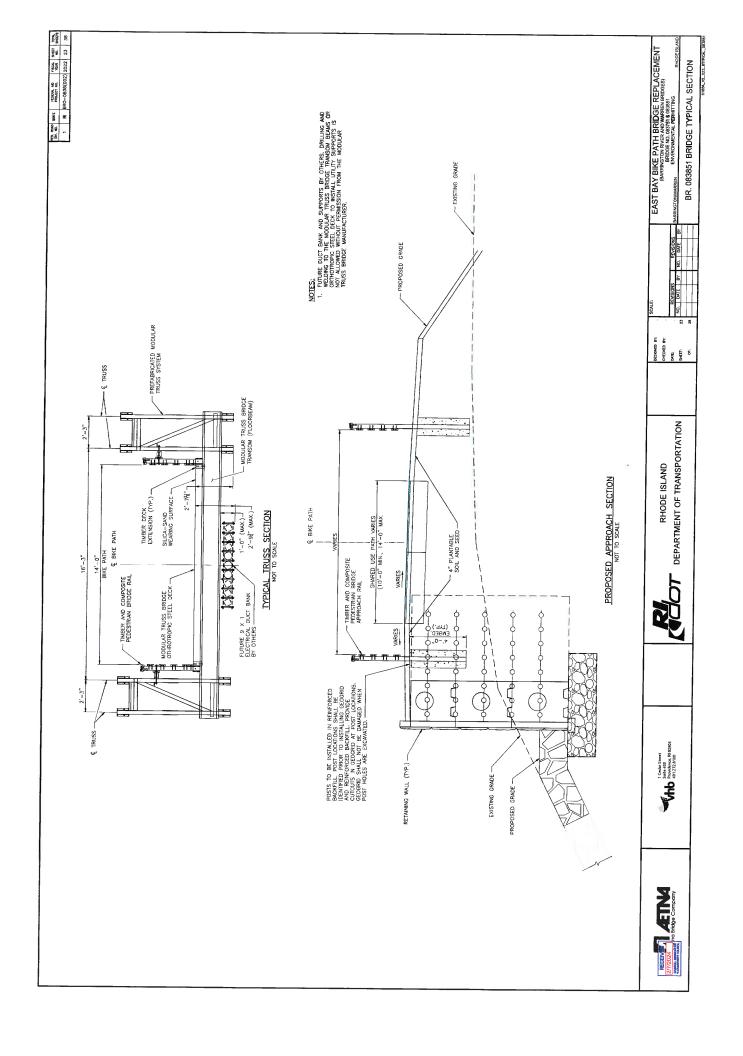




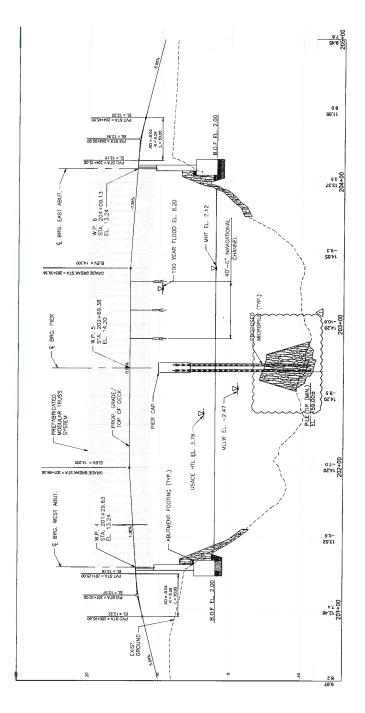








7.



B WARREN PROFILE SCALE HORIZONTAL: 1"=20' SCALE VERTICAL: 1"=4"

EAST BAY BIKE PATH BRIDGE REPLACEMENT
(SARBINGTON RIVER AND WARRELINGGES)
(BARBINGENO 68775, 8 058567
(BANTROWNER) PERMITTING
REDORMENTAL PERMITTING

NOTE: REFER TO HIGHWAY PLANS FOR ADDITIONAL INFORMATION.

BR. 083851 BRIDGE PROFILE





OT DEPARTMENT OF TRANSPORTATION

RHODE ISLAND



